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DEVELOPMENT AND EXPERIMENTAL VERI-FICATION OF PROCEDURES TO DETERMINE NONLINE AR LOAD-DEFLECTION CHARAC TERISTICS OF HELICOPTER SUBSTRUCTURES SUBJECTED TO CRASH FORCES. VOLUME II. TEST DATA AND DESCRIPTION OF REFINED PROGRAM 'KRASH', INCLUDING A USER'S GUIDE AND SAMPLE CASE

G. Wittlin, et al

Lockheed-California Company

Prepared for:

Army Air Mobility Research and Development Laboratory

May 1974

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SECURITY CLASSIFICATION OF THIS PAGE (Then Date Ministed)

REPORT DOCUMENTATION PAGE		READ INSTRUCTIONS BEFORE COMPLETING FORM
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USAAMRDL-TR-74-12B		AI) 784 192
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7. AUTHOR(a)	WCEO - AODONE 11	8. CONTRACT OR GRANT NUMBER(+)
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9. PERFORMING ORGANIZATION NAME AND ADDRESS		10. PROGRAM ELEMENT, PROJECT, TASK AREA & WORK UNIT NUMBERS
Lockheed-California Company		Task 1F162205AH88
Burbank, California 91503		
11. CONTROLLING OFFICE NAME AND ADDRESS		12. REPORT DATE
Eustis Directorate		May 1974
U. S. Army Air Mobility R&D Labo Fort Eustis, Virginia 23604	ratory	13. NUMBER OF PAGES
14. MONITORING AGENCY NAME & ADDRESS(If differen	it from Controlling Office)	18. SECURITY CLASS. (of this report)
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IS. SUPPLEMENTARY NOTES		
Volume II of a two-volume report		
19. KEY WORDS (Continue on reverse side if i.ecessary at		
Aircraft Tolerances (physiology) Crash resistance Helicopters		
Dynamics Energy absorbers		
Structural properties Mathematical models		
20. ABSTRACT (Continue on reverse side if necessary and identify by block number)		
The results of a study to develo		
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structures are presented. A literature survey is performed in which 60 technical reports and papers are evaluated with regard to their applicabil-

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Studies using an existing 31 lumped mass model of the UH-IH helicopter are performed to determine the sensitivity of responses to changes in the load-deflection representation for the engine and transmission mounts, landing gear and fuselage. Simplified techniques are used to predict the load-deflection curve for the crushing of a segment of the lower fuselage under impact conditions. The predictions include elastic behavior, failure load and post-failure behavior. The structural segment selected for analysis and test is a section supported on four edges, representative of the transmission pylon support.

Twelve specimens were fabricated. The specimens are 46 inches long by 18 inches wide by 6.125 inches to 12.125 inches deep. The 6.125-inch-deep specimens are approximately half the size (except for thickness) and are varied in detail design (number of angles, spacing of angles, lightening holes). Static and dynamic tests were performed. The predicted load-deflection curves are compared to the test load-deflection curves and show good agreement with regard to peak failure load, failure point, energy absorbed, and shape. The results of the tests show that for this type of typical fuselage structure, static tests provide load-deflection data which is similar to data that can be obtained from dynamic tests, but more economically.

Program KRASH is refined to facilitate its use by designers. In particular, the input data is reordered, some inputs are standardized and more general load-deflection curve characteristics are possible. The capacity of the program is increased to 80 lumped masses, 100 internal beams and 120 load-deflection tables. The refined program was run to demonstrate capability to treat a three-dimensional impact velocity, mass penetration into an occupiable space, and simplified rotor blade contact. Specimen test data is also used to refine the 31 mass UH-1H model.

The analytical techniques developed herein are presented in the form of design charts, nomographs, curves, tables and equations and form the basis of a structural crashworthiness design manual. The design procedures are outlined in a step-by-step process including examples.

Volume II contains supporting analytical and test data and a literature matrix categorization. A description of refined program KRASH is provided which shows the new input-output format, a listing, and sample problems.

EUSTIS DIRECTORATE POSITION STATEMENT

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The Eustis Directorate technical monitor for this effort was Mr. G. T. Singley III of the Military Operations Technology Division.

The conclusions submitted by the contractor are considered to be valid.

The report is divided into two volumes. Volume I contains a description of the survey of technical publications, investigation of the sensitivity of the simulated structural response to load-deflection variations, substructure test program, refinement of KRASH, structural crashworthiness design procedures, and results obtained. Volume II contains abstracts of literature reviewed, supporting analytical and test data, a description of the refined KRASH computer program, and a user's guide for the computer program.

DISCLAIMERS

The findings in this report are not to be construed as an official Department of the Army position unless so designessed by other authorized documents.

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INTRODUCTION

This volume contains test and analysis data in support of the detailed discussions presented in Volume I. The 60 technical reports and publications reviewed during the study are briefly summarized and then tabulated in a Literature Survey Subject Index. The test data is presented in the form of recorded load, deflection, strain and accelerometer readings versus scan (time). The refinements of program KRASH to facilitiate its usage are described. Included in the writeup for program KRASH is the refined input-output format, user's guide, listing, and two sample problems. Also included in the section on program KRASH are the energy balance equations and the sample format for the energy data.

LITERATURE SURVEY AND SYNOPSIS

1. Wittlin, G., Gamon, M.A., EXPERIMENTAL PROGRAM FOR THE DEVELOPMENT OF IMPROVED RELICOPTER STRUCTURAL CRASHWORTHINESS ANALYTICAL AND DESIGN TECHNIQUES, Lockheed-California Company; USAAMRDL Technical Report 72-72A,-72B, Eustis Directorate, U.S. Army Air Mobility Research and Development Laboratory, Fort Eustis, Virginia, May 1973, AD 764985.

This report presents the results of a four-phase study to develop improved rotary-wing aircraft structural crashworthiness analytical and design techniques. A digital computer program, designated KRASH, was developed and shown to be capable of accurately predicting responses during a crash in which multidirectional forces are present. The program was verified using controlled test data obtained on the same program via a combined vertical-lateral velocity impact using a fully instrumented UH-1H helicopter. Farameter studies were performed to ascertain the effect on the response of the structure and the occupants due to changes in structural element load deflection characteristics. A consistent design approach is presented, and the results of the parameter study are used to illustrate its application in a crash analysis. The study also included a detailed accident investigation, a literature survey and evaluation, substructure test, and analysis.

2. Turnbow; J.W., Carroll, D.F., Haley, J.L., Jr., Robertson, S.N., CRASH SURVIVAL DESIGN GUIDE. Dynamic Science; USAAMRDL Technical Report 71-22, U.S. Army Air Mobility Research and Development Laboratory, Fort Eustis, Virginia, October 1971, AD 733358.

This report is a design guide that has been assembled to provide the engineer with an understanding of the basic problems associated with the development of crashworthy U.S. Army aircraft. Where possible, solutions to specific problems are indicated. In areas in which little design data are available, only the general philosophy appropriate to the problem solution is presented; the details of such solutions as well as the degree of crashworthiness to be achieved must be left, at present, to the ingenuity of the designer.

This guide presents, in a condensed form, the data, design techniques, and criteria that are presently available in eight areas: (1) aircraft crash kinematic and survival envelope, (2) airframe crashworthiness design criteria, (3) aircraft seat design criteria (crew and troop/passenger), (4) restraint system design criteria (crew, troop/passenger, and cargo), (5) occupant environment design criteria, (6) aircraft ancillary equipment stowage design criteria, (7) emergency escape provisions, and (8) postcrash fire design criteria.

It is intended that both airframe and component designers and manufacturers use this guide to extend the "region of survivability" in aircraft accidents to a maximum level.

3. Reed, William H., Avery, James P., Ph.D, PRINCIPLES FOR IMPROVING STRUCTURAL CRASHWORTHINESS FOR STOL AND CTOL AIRCRAFT, Aviation Safety Engineering and Research; USAAVIABS Technical Report 66-39, U.S. Army Aviation Materiel Laboratories, Fort Eustis, Virginia, June 1966, AD 637133.

In this report the crash behavior of aircraft structures is investigated. The investigation begins with the definition of two indices of crashworthiness of basic aircraft structures and the analysis of the influence of several general types of structural modifications upon these two indices. This analysis, using fundamental principles of mechanics, contains several simplifying assumptions, which are explained as they are introduced.

Design concepts to improve the ability of the "protective container" to maintain living space for occupants during a crash or to attenuate the accelerations experienced by occupants during a crash are developed for crash conditions which are either primarily longitudinal in nature or primarily vertical in nature. Analytical methods are then provided to show how and when to apply these design concepts to any particular aircraft. Principles are presented which are considered to be suitable for use during design of new aircraft as well as modifications of existing aircraft.

The results are presented from three full-scale crash tests of small twin-engine airplanes which were conducted as a part of this investigation.

Among the pertinent conclusions of the report are: (1) improvements in crashworthiness can be achieved via minor changes in structural design or modification of existing structure, (2) vertical and longitudinal impact environments offer significantly different problems in designing for improved crashworthiness, and (3) analysis of aircraft behavior is hampered by the lack of adequate knowledge of the relationships which apply to the determination of the reaction force which decelerates the aircraft upon contact with the ground.

4. Greer, D.L., et al, DESIGN STUDY AND MODEL STRUCTURES TEST PROGRAM TO IMPROVE FUSELAGE CRASHWORTHINESS, General Dynamics, Convair; FAA Report DS-67-20, Federal Aviation Administration, Washington, D.C., October 1967, AD 666816.

This report presents the results of a study to evaluate methods to improve crashworthiness by retaining transport cabin integrity during crash impact loadings.

The study includes analyses of the effects of strengthening, redistribution of bending material, and incorporation of energy dissipating features on the ability of the fuselage to provide a protective shell around the occupants. Analytical results were substantiated by a test program.

The test program included compression tests of plate-stringer panels and drop tests of representative fuselage structure. Tests were made on three 100-inch-diameter cylindrical sections dropped axially, four segments of 100-inch-diameter cylinders dropped laterally, and a structurally comple nose section of a jet transport dropped in a 10-degree nose-down attitude.

The requirement for a plastically deforming structure is important for both axial and vertical collapse characteristics of a fuselage. Plastic collapse provides the most efficient energy-absorbing capability and also reduces the possibility of excessive tearing or complete disintegration of the structure.

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Both energy-absorbing capacity and failure mode are important for vertical crushing conditions. The recommended manner of reinforcement for the fuselage lower frame segments strengthens the bottom centerline portion and the floor beam to frame area. No significant weight or cost penalty is involved since the crash requirement reinforcement occurs at the most critical areas for existing design conditions.

5. Saczalski, K.J., and Park, K.C., TRANSIENT RESPONSE OF INELASTICALLY CONSTRAINED RIGID BODY SYSTEMS, to appear in the <u>Journal of Engineering for Industry</u>, A.S.M.E. Transactions, 1974.

An energy rate balance is employed to develop the incremental equations of motion for a shock-loaded, inelastically constrained, rigid-body structural system. Lagrangian multipliers provide the coupling mechanism necessary to reduce the overall system of equations to a set of modified rigid-body equations which include the nonlinear geometric and structural material effects. Kinematic material hardening and a modified yield criteria are used. Examples illustrate the technique and are compared with experimental results.

6. Massonnet, C.E., and Save, M.A., PLASTIC ANALYSIS AND DESIGN, VOL. I, BEAMS AND FRAMES; Blusdell, New York, 1965.

This book deals with the analysis and design of beams and frames made of a ductile material on the basis of the ultimate load. With the second volume, which is concerned with more complicated structures such as plates and shells, it aims at giving a broad (if not exhaustive) coverage of plastic analysis and design methods. These methods essentially apply to mild steel structures, but may also be used, with adequate caution, for reinforced and prestressed concrete structures.

Presently, the first volume is the most important for practical applications. The so-called simple plastic theory of beams and frames is

presented in Volume II, which is now being finalized.

The material has been considered as an engineering problem and not as applied mathematics. Much attention and extensive treatment have been given to plastic buckling and to design of joints. The influence of details of construction (machining, drilling, punching, etc.) is also considered.

7. Roark, R.J., FORMULAS FOR STRESS AND STRAIN, McGraw-Hill, New York, 1965.

This book provides a compact, complete summary of the formulas, facts, and principles pertaining to strength of materials. It is intended primarily as a reference book and represents an attempt to meet what is believed to be a present need of the design engineer.

Presented are certain general principles. Included are brief descriptions of analytical and experimental methods of stress analysis and information concerning the behavior of material under stress. The behavior of structural elements under various conditions of loading is discussed, and extensive tables of formulas for the calculation of stress, strain, and strength are given. Derivations and detailed explanations are omitted. However, examples are included to illustrate the application of the various formulas and methods.

8. Ayre, Robert S., Shock and Vibration Handbook, Chap. 8, Vol. 1, TRANSIENT RESPONSE TO STEP AND PULSE FUNCTIONS, McGraw-Hill, 1961.

Hoppmann, W.K., Shock and Vibration Handbook, Chap. 9, Vol. 1, EFFECTS OF IMPACT ON STRUCTURES, McGraw-Hill, New York, 1961.

Chapter 8 deals briefly with methods of analysis for obtaining the response spectrum from the time history, and includes in graphical form certain significant spectra for various regular step- and pulse-type excitations. The usual concept of the response spectrum is based upon the single-degree-of-freedom system, usually considered linear and undamped although useful information sometimes can be obtained by introducing nonlinearity or damping. The single-degree-of-freedom system is considered to be subjected to the shock or transient vibration, and its response is determined.

The response spectrum is a graphical presentation of a reflected quantity in the response taken with reference to a quantity in the excitation. It is plotted as a function of a dimensionless parameter that includes the natural period of the responding system and a significant period of the excitation. The excitation may be defined in terms of various physical quantities, and the response spectrum likewise may depict various characteristics of the response.

Chapter 9 discusses a particular phenomenon in the general field of shock and vibration usually referred to as impact. An impact occurs when two or more bodies collide. An important characteristic of an impact is the generation of relatively large forces at points of contact for relatively short periods of time. Such forces sometimes are referred to as impulse-type forces.

Three general classes of impact are considered in this chapter: (1) impact between spheres or other rigid bodies, where a body is considered to be rigid if its dimensions are large relative to the wavelengths of the elastic stress waves in the body; (2) impact of a rigid body against a beam or plate that remains substantially elastic during the impact; and (3) impact involving yielding of structures.

9. Timoshenko, S. Woinowsky-Krieger, S., THEORY OF PLATES AND SHELLS, McGraw-Hill Publishers, New York, 1959.

This book deals with the three regions of plate and shell theory: (1) thin plates with small deflections; (2) thin plates with large deflections; (3) thick plates. The book considers problems with membrane stresses and the case with clamped edges. Simplifications are given for special cases of deformation to the shape of a developable surface. The thick plate theory presented considers the problem as a three-dimensional problem of elasticity.

10. Tulk, F.D., BUCKLING OF CIRCULAR CYLINDRICAL SHELLS UNDER DYNAMICALLY APPLIED AXIAL LOADS, UTTAS report 160, 1972.

A theoretical and experimental study was made of the buckling characteristics of perfect and imperfect circular cylindrical shells subjected to dynamic axial loading.

The tests were performed on a specially designed dynamic testing machine which was capable of producing controlled ramp-type loads at rates ranging from the quasi-static up to higher than 200,000 pounds/second. Ten shell specimens were tested, including two nearperfect shells, seven shells with axisymmetric sinusoidal imperfections of a variety of amplitudes and wavelengths, and one shell with quasi-random axisymmetric imperfections. The shells were produced from a photoelastic epoxy plastic using a spin-casting technique. The imperfection profiles were machined into the shell walls using a high-precision hydraulic tracing apparatus. For three of the shells with sinusoidal imperfections, imperfection profiles were cut on the inner surface alone; while for the remaining four shells with sinusoidal imperfections and for the shell with quasi-random imperfections, a special manufacturing procedure was adopted which produced shells with matching inner and outer profiles, thus providing effectively constant thickness walls.

Experimental data included dynamic buckling loads (124 data points), high-speed photographs of the buckling mode shapes, and observations of the dynamic stability of shells subjected to rapidly applied subcritical loads.

A mathematical model is developed to describe the dynamic behavior of perfect and imperfect shells. This model is based on the Donnell-von Karman compatibility and equilibrium equations and has a wall deflection function incorporating five separate modes of deflection. Close agreement between theory and experiment is found for both dynamic buckling strength and buckling mode shapes.

11. Stricklin, J.E., et al, IARGE DEFLECTION ELASTIC-PLASTIC DYNAMIC RESPONSE OF STIFFENED SHELLS OF REVOLUTION, TEES-RPT-72-25 and SLA-73-0128, 1972.

This report presents the formulation and check-out problems for a computer code DYN.PIAS, which analyzes the large deflection elastic-plastic dynamic response of stiffened shells of revolution. The formulation is by the finite element method, with finite differences being used for the evaluation of the pseudo forces due to material and geometric nonlinearities. Time integration is by the Houbolt method. The stiffness may be due to concentrated or distributed eccentric rings and spring supports at arbitrary angles around the circumference of the elements. Check-out problems include the comparison of solutions from DYNAPIAS with experimental and other computer calculations for rings, conical and cylindrical shells, and a curved panel. A hypothetical submarine including stiffeners and missile tube is studied under a combination of hydrostatic and dynamically applied asymmetrical pressure loadings.

12. Becker, H., and Gerard G., HANDBOOKS OF STRUCTURAL STABILITY, PARTS I-V, NACA Technical Notes 3781-3785, 1957.

The local buckling of stiffener sections and the buckling of plates with angle stiffeners are reviewed, and the results are summarized in charts and tables. Numerical values of buckling coefficients are presented for longitudinally compressed stiffener sections of various shapes, and for stiffened cylinders loaded in torsion. Although the data presented consists primarily of elastic-buckling coefficients, the effects of plasticity are discussed for a few special cases.

13. Skogh, J., Stern, P., POSTBUCKLING BEHAVIOR OF A SECTION REPRESENTATIVE OF THE B-1 AFT INTERMEDIATE FUSELAGE, Lockheed Palto Alto Research Laboratory, AFFDL-TR-73-03.Air Force Flight Dynamics Laboratory, Air Force Systems Command, Wright-Patterson Air Force Base, Ohio, May 1973.

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A section of the B-l aft intermediate fuselage consisting of a combination of flat and curved panels is analyzed for postbuckling strength under a combination of torque and axial loading. The analysis, which extends to load levels about ten times the load that produces the first buckle, was carried out rigorously by the use of the finite-difference computer code STAGS. The results show that the fuselage section does not collapse at the applied ultimate load. Shear stiffness values as a function of the applied load are calculated. These data can be used as inputs for a finite-element analysis of the fuselage section.

14. Atluri, S., PETROS 3: A FINITE-DIFFERENCE METHOD AND PROGRAM FOR THE CALCULATION OF LARGE ELASTIC-PLASTIC DYNAMICALLY-INDUCED DEFORMATIONS OF MULTILAYER VARIABLE THICKNESS SHELLS, BRL, U.S. Army Aberdeen Research & Development Center, Aberdeen Proving Ground, Maryland, Contract #DAADO5-68-C-0314, Nov. 1971.

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The governing equations for the arbitrarily large-deformation elastic-plastic transient responses of variable-thickness, hard-bonded, multi-layer, multimaterial, thin, Kirchhoff shells of any initial shape are formulated and solved by the finite-difference technique. The material is assumed to be initially isotropic and to exhibit elastic, strain-hardening, strain-rate-sensitive, and temperature-dependent behavior. The structure may be subjected to a variety of initial velocity distributions, transient mechanical loads, and/or transient thermal loads. These capabilities and features are contained in a computer program, PETROS 3, which has been applied to a variety of example problems.

Included is a FORTRAN IV listing and a description of PETROS 3 together with the data input and solution output for several example problems.

15. Haftka, R.T., A KOITER-TYPE METHOD FOR FINITE ELEMENT ANALYSIS OF NONLINEAR STRUCTURAL BEHAVIOR, AFFDL-TR-70-130, Vol. I, Air Force Flight Dynamics Laboratory, Wright-Patterson Air Force Base, Ohio, Nov. 1970.

Koiter's method for the asymptotic analysis of post-buckling behavior is reformulated in finite element notation for application to structures idealized by finite element models. Originally restricted to the analysis either of structures exhibiting bifurcation buckling or of slightly imperfect versions of such structures, Koiter's method is therein adapted to a more general class of structures exhibiting the more common snap-through (limit point) type of buckling. This adaption of Koiter's method is referred to as the Modified Structure method. It is accomplished by modification of the actual energy functional to create a hypothetical modified structure having a strictly linear pre-buckling path along which buckling must be of

the bifurcation type. The analysis of the actual structure is then accomplished by application of Koiter's method through consideration of the actual structure as an imperfect version of the modified structure.

In this way, the Modified Structure method operates within the theoretical framework established by Koiter, and the effects of prebuckling nonlinearity are approximated asymptotically. Various levels of approximation are considered. Additionally, the use of the Modified Structure method, in conjunction with direct methods of nonlinear analysis, is examined. A highly accurate finite element representation is employed in presenting a comprehensive numerical evaluation of the Modified Structure method of analysis on the basis of a number of planar frame problems. Collectively, these examples exhibit a broad spectrum of nonlinear behavior characteristics. Emphasis throughout is placed upon assessing the limitations and attributes of the Modified Structure method of analysis. Conclusions regarding applicability and performance emerge from detailed examination of the results obtained.

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16. Stilwell, W.C., and Ball, R.E., A DIGITAL COMPUTER STUDY OF THE BUCKLING OF SHALLOW SPHERICAL CAPS AND TRUNCATED HEMISPHERES, NASA CR 1998, June 1972.

Several user-oriented digital computer programs for the static analysis of shells of revolution exist. A detailed discussion of most of these programs is given in Reference 1 of the report. Of particular interest here is the program developed by Ball (Reference 2) for the geometrically nonlinear analysis of arbitrarily loaded shells of revolution. This program is an equilibrium program; that is, it solves for the displacement and stress resultant fields for an arbitrary loading condition. Since geometric nonlinearities are included, the magnitude of load that leads to a condition of instability can be determined.

The utility of the program would be considerably enhanced if it could be used to determine bifurcation buckling loads and the behavior of the shell in the vicinity of the bifurcation load. This latter feature is often referred to as the imperfection sensitivity of the shell to the load. As a consequence, the objective of this study was to use the computer program to examine the buckling behavior of several shells subjected to axisymmetric and nearly axisymmetric loads. It was anticipated that an examination of the effects of the small asymmetric perturbations upon the stability of the shell would disclose the bifurcation buckling load and provide a quantitative evaluation of the imperfection sensitivity of the shell to the load.

17. Witmer, E.A., LARGE DYNAMIC DEFORMATIONS OF BEAMS, RINGS, PLATES AND SHELLS, ALAA Journal, Vol. I, No. 2, August 1963.

The axisymmetric responses of shells, plates, rings, and beams to impulsive or blast loading that produces large deformations involving both the elastic and plastic regions of material behavior are analyzed. A general numerical method that includes (1) elastic, (2) perfectly plastic, (3) elastic, strain-hardening, and/or (4) elastic, strain-hardening, strain-rate sensitive material behavior and large structural deflections is formulated and applied. In the timewise st: by-step numerical analysis, the increments in stress resultants and stress couples are determined by idealizing the shell thickness as consisting of n concentrated layers of materials separated by a material that cannot carry normal stresses but has infinite shear rigidity. The influences of the number of layers employed in the idealized model, as well as the aforementioned various types of material behavior, are demonstrated. Theoretical predictions of time-history responses and/or final structural deformations are compared with experimental data for impact-loaded spherical shells, for blast-loaded circular plates, and for explosively loaded circular rings and clamped beams.

18. Perrone, N., ON A SIMPLIFIED METHOD FOR SOLVING IMPULSIVELY LOADED STRUCTURES OF RATE-SENSITIVE MATERIALS, Office of Naval Research, Washington, D.C., Journal of Applied Mechanics, A.S.M.E., September 1965.

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In an attempt to assess more completely rate-sensitivic material effects, two fundamental structural elements are analyzed: a wire with an impulsively loaded end mass, and an impulsively loaded ring. The ring and wire are made of perfectly plastic, rate-sensitive materials. In each case, exact and approximate solutions are obtained for an exponential rate-sensitivity law. The results suggest that very good approximations to the exact solutions may be found by utilizing a rate-insensitive material with constant yield stress equal to the initial dynamic yield stress.

19. Bodner, S.R., and Symonds, P.S., EXPERIMENTAL AND THEORETICAL IN-VESTIGATION OF THE PLASTIC DEFORMATION OF CANTILEVER BEAMS SUBJECTED TO IMPULSIVE LOADING, Brown University, Journal of Applied Mechanics, December 1962.

The experimental techniques and the results obtained in a program to evaluate the assumptions of dynamic, rigid-plastic theory of beams are presented. The experiments use steel and aluminum-alloy cantilever beams subjected to either a rapid velocity change at the base or to an impulsive load at the tip. A rigid-plastic theory that includes the strain-rate dependence of the yield stress and geometry changes is outlined for the case of the tip impulsive loading. Predictions of this theory are in satisfactory agreement with the experimental results.

20. Hibbit, H.D., et al, A FINITE ELEMENT FORUMLATION FOR PROBLEMS OF LARGE STRAIN AND LARGE DISPLACEMENT, Brown University, Int'l Journal of Solids and Structures, 1970, Vol 6, pp. 1069-1086.

An incremental and piecewise linear finite element theory is developed for the large-displacement, large-strain regime with particular reference to elastic-plastic behavior in metals. The resulting equations, though more complex, are in a similar form to those previously developed for large-displacement, small-strain problems, the only additional term being an initial load stiffness matrix which is dependent on current loads. This similarity in form means that existing nonlinear general-purpose programs may easily be extended to include finite strains. A large-displacement, small-strain formulation (as applicable to problems of structural stability) is obtained from this theory by assuming that changes in length of line elements and relative rotation of orthogonal line elements are negligible compared to unity. The simplified equations are in essential agreement with previous formulations in the literature. The only difference which is observed is the persistence of the initial load stiffness matrix, which may be significant in some cases.

21. Toridio, T.G., and Khozeimeh, K., INELASTIC RESPONSE OF FRAMES TO DYNAMIC LOADS, Journal of Engineering Mechanics, June 1971.

A procedure of analysis is presented for determining the elastic-inelastic response of framed structures under dynamic loads. Application of Hamilton's principle in conjunction with the finite-element method leads to the basic dynamic equation of the system incorporating the plastic effects in the form of equivalent nodal forces. This approach also allows the more accurate treatment of the distributed mass of the element than the usual geometrical method of lumping.

22. McDaniel, T.J., DYNAMICS OF STIFFENED CYLINDRICAL SHELLS WITH SPATI-ALLY VARYING CUPVATURE, University of Dayton Research Institute, Air Force Materials Laboratory Report AFML-TR-72-134, Air Force Systems Command, Wright-Patterson Air Force Base, Ohio, July 1972.

This report presents the results of a theoretical study of the effects of spatially varying curvature on the dynamics of cylindrical panels and stiffened cylindrical shell structures. The first section of the report contains a brief discussion of the general problem area. Following this discussion, the analytical techniques for solving constant-curvature cylindrical panels and stiffened cylindrical shell problems by a transfer matrix approach are reviewed. These techniques are found to apply directly to the varying curvature shell analysis, provided the transfer matrix for this type of shell can be obtained. An analytical approach to obtaining the transfer matrix for a shell with varying curvature is explored. A solution to the transfer matrix for a cylindrical shell with exponentially varying curvature is obtained. In a following section, the preparation of this solution to obtain numerical transfer matrix is discussed. Several approximate and numerical

procedures for obtaining a transfer matrix are explored. Finally, the dynamic responses of both single panels and stringer stiffened cylindrical structures with increasing and decreasing curvature are compared to similar structures with constant curvature.

23. Bendix Corp., Final Engineering Report, ENERGY ABSORBING CHARACTER-ISTICS OF CRUSHABLE ALUMINUM STRUCTURES IN A SPACE ENVIRONMENT, NAS -CR-65096, July 1965.

The research effort detailed in this report involves the basic objective of obtaining quantitative design data concerning the characteristics of aluminum honeycomb materials when used in high L/D ratio, crushable, energy-absorbing capsules. Nine configurations of honeycomb energy-absorbing capsules using alloy 5056 are evaluated. The nine basic configurations incorporate three cross-sectional shapes of high, medium, and low crush strength, each of which was fabricated with cell axes oriented at angles of 0, 15 and 30 degrees to the capsule longitudinal axis. The characteristics which are studied included specific energy, load onset rate, and rebound. Variations of these characteristics are investigated under controlled environmental conditions.

The capsules were subjected to both static and dynamic loads, impact velocities from 5 thru 20 feet per second, and impact weights varying from 760 thru 3750 pounds. The environmental extremes under which the specimens were tested spanned the temperature range from -260° F thru room temperature up to $+300^{\circ}$ F, and a vacuum of 3 x 10^{-1} TORR.

24. Kornhauser, M., STRUCTURAL EFFECTS OF IMPACT, Sparton Books, Inc., Baltimore, Md., 1964.

Mechanical impact has been treated traditionally, in the United States, as a subject which is variously appended to college textbooks on elasticity, strength of materials, engineering mechanics, or vibrations. The elasticity and strength of materials texts are generally concerned with local surface effects or with stress waves; the mechanics texts usually treat the whole-body motions on impact; and the vibrations texts are apt to consider impact a special case of unsteady vibrations and amplification factors. In the actual case of impact, all of these phenomena and effects come into play, and the significance of each effect must be emphasized relative to the purpose of the analysis.

In this book the emphasis is placed on go or no-go behavior, survivability, or failure. Loading and response must, or course, be analyzed. Nevertheless, wherever possible, the object is to permit estimates of failure directly in terms of the loading conditions. Impact is a complex process. Given the loading history of a structure (and assuming no interaction with the structure's response), it is, in general, impossible to trace the stress waves and their

reflections throughout the structure, resulting in vibrations and permanent set or failure. Many solutions of loading and response, as well as theories of failure in terms of material properties, are available in the literature for various idealized conditions and configurations. What is desired by the practicing engineer are some relatively simple approaches to prediction of failure. This book is intended as a start in this direction.

In attempting to provide engineering answers to some very complex problems, liberal use has neen made of "engineering judgment" and idealizations. The soundness of each assumption is in proportion to the number of exact theoretical and experimental results available for very similar situations. For this reason it is anticipated that modifications of the approaches presented therein will be appropriate as the various disciplines produce more data.

The book is organized in three sections: "Loading Conditions", "Response and Failure of Structures", and "Effects of Impact." The section on response and failure is expository in nature, introducing the background theory and application of the sensitivity method of presenting and predicting inertial failure, as well as discussing low-speed and hypervelocity impact effects. Having accepted the approaches resommended in the second section, the practicing engineer may use the curves and tables of the first and third sections for prediction of impact effects in terms of the environmental input functions.

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25. Fisher, L. J., Jr., LANDING-IMPACT-DISSIPATION SYSTEMS, NASA, Technical Note D-975, December 1961.

Analytical and experimental investigations are made to determine the landing-energy-dissipation characteristics for several types of earth-landing-impact systems having application to reentry vehicles. The areas of study are divided into three velocity regions: (1) those having primarily vertical velocity, (2) those having both moderate horizontal and moderate vertical velocity, and (3) those having primarily horizontal velocity. The impact systems discussed are braking rockets, gas-filled bags, frangible metal tubing, aluminum hone; comb, balsa wood, strain straps, and both skid and skid-rocker landings on hard-surface runways and on water.

The report states that it appears feasible to evaluate landing-gear systems for reentry vehicles by computational methods and free-body landing techniques with energy dissipation for an earth landing of such a vehicle. Some systems are more efficient than others, some package better than others, and a variety of promising systems are under study. Horizontal energy dissipation is simpler to deal with than vertical energy dissipation since translational friction is all that is involved; however, runout behavior becomes a factor. Vertical velocity can also be a big factor when high flight-path angles are

associated with even moderate horizontal velocities. High-speed landings are particularly a problem, especially high-speed water landings, and indications are that if large horizontal velocities are involved in hard-surface landings, a selected site will be required.

26. McGehee, J. R., A PRELIMINARY EXPERIMENTAL INVESTIGATION OF AN ENERGY-ABSORPTION PROCESS EMPLOYING FRANGIBLE METAL TUBING, NASA Technical Note D-1477, 1962.

A highly efficient energy-absorption process, employing frangible metal tubing as the working element, is investigated. A preliminary experimental investigation is conducted to determine the variation of the average fragmenting stress of 2024-T3 aluminum-alloy tubing with the pertinent parameters of this process. Limited tests were made to determine the feasibility of employing this process in a landing-gear system. A 1/5-scale model of a proposed manned spacecraft with a landing gear incorporating this process is employed in these tests.

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The results of this investigation show that the fragmenting process produces a fluctuating force with displacement, but for a fixed set of parameters, the force about which the fluctuation occurs is approximately constant. A large force which occurs when the process is started with the unaltered tube seated symmetrically in the die can be reduced most effectively by tapering the wall thickness over a short length at the die end of the tube. The average fragmenting stress, for 2024-T3 aluminum-alloy tubing and the range of parameters investigated, appears to be independent of the ratio of wall thickness to tube diameter, but varies as the cube of the ratio of the wall thickness to the radius of the forming die. The fragmenting stress obtained at 12,000 inches per minute was about 60 percent higher than those obtained at 1 inch per minute. The 2024-T3 aluminum-alloy tubing, when fragmenting on a die at 90 percent of the yield stress, is capable of absorbing 31,000 foot-pounds of energy per pound of material. This energy-absorption capability is greater than that of the most frequently considered processes; for example, the crushing of balsa wood, aluminum honeycomb, or pressurized thin-walled metallic cylinders. Model tests, employing frangible tubing as the working element in the landing gear, indicate that this process is suitable for use in a load-alleviation application.

27. Kroell, C. K., A SIMPLE, EFFICIENT, ONE SHOT ENERGY ABSORBER, General Motors Research Laboratory, Warren, Michigan, Shock, Vibration and Associated Environments, Part III, Bulletin No. 30, 1962.

This paper describes a single-shot expendable energy absorber which has been developed at the General Motors Research Laboratory. The device is inherently simple and is characterized by a rectangular force-displacement relationship and high specific energy absorption capacity. Both a qualitative discussion of the mechanics of the

plastic deformation process involved and a graphical summary of the experimental performance data which have been collected to date are presented.

28. Weinberg, L.W.T., and Turnbow, J.W., Ph.D., SURVIVABILITY SEAT DESIGN DYNAMIC TEST PROGRAM, Aviation Safety Engineering and Research, USAAVIABS Technical Report 65-43, U.S. Army Aviation Materiel Laboratories, Fort Eustis, Virginia, July 1965, AD 621718.

This report presents the results of a series of dynamic tests conducted with four different concepts of experimental cre seats.

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The experimental seats were designed and constructed by four helicopter manufacturers. The seats were designed to withstand static load factors equivalent to those recommended in TRECOM Technical Report 63-4, "Crew Seat Design Criteria for Army Aircraft", dated February 1963.

The design load factors recommended in the above-referenced report are as follows: longitudinal - 45G for 0.10 second; lateral - 45G for 0.10 second; and vertical - 25G for 0.10 second.

Special kits for small-arms ballistic protection were also designed and installed in the seats tested.

These seats were designed exclusively using static load factors. No previous testing was conducted by any seat manufacturer prior to the conduct of these tests.

The four seats were tested under four load conditions. Two of the conditions involved simultaneous half loads on the seats in two different seat positions, and two of the conditions involved full loads in two different seat positions.

Only one of the four seats tested withstood the loads imposed for all four conditions. Three of the seats failed and were damaged beyond economical repair when each was subjected to the first full-load test condition.

This report also includes a detailed description of an acceleration device which was specifically designed and fabricated for this series of tests.

29. Langhaar, H. L., THEORETICAL AND EXPERIMENTAL INVESTIGATIONS OF THIN-WEBBED PLATE-GIRDER BEAMS, Transactions of the ASME, October 1943.

A simple, semirational theory for the design of webs and flange rivets of thin-webbed rectangular plate-girder shear beams is presented. Calculations of shear loads to cause web rupture and flange rivet failure are compared with test data from 27 beams.

30. Perry, D. J., AIRCRAFT STRUCTURES, McGraw Hill Book Co., New York, 1950.

In this book an attempt is made to emphasize basic structural theory which will not change as new materials and new construction methods are developed. Most of the theory is applicable for any design requirements and for any materials. The design engineer may then supplement this theory with the detail design specifications and the material properties which are applicable to his particular airplane.

Heavy emphasis is placed on the application of the elementary principles of mechanics to the analysis of aircraft structures.

31. Jones, N., et al, THE DYNAMIC PLASTIC BEHAVIOR OF SHELLS, MIT Report 71-6, 1971.

A survey is made of the literature published previously on the inelastic behavior of shells subjected to dynamic loads. An experimental investigation is also being undertaken to examine the behavior of various cylindrical shell panels which are loaded with an impulse on the inner surface. The panels are fully clamped along the two longitudinal edges and free on the other two. The initial kinetic energy of the dynamic loads is sufficiently large to cause inelastic behavior and to produce maximum permanent transverse deflections of up to nearly twice the corresponding panel thickness. Tests are conducted on mild steel and aluminum 6061-T6 panels which have various thicknesses and included angle of 90° approximately.

32. Mitchell, B., THE DYNASORB ENERGY ABSORBER, Lockheed Report LR 16735, March 1963.

This report introduces a method of absorbing energy while maintaining a constant load level. The tubular load-carrying member can support its full load capability and still be almost totally consumed by rolling up one end. The energy curves presented are typical and show excellent load consistency with efficiencies as high as 450,000 in.—1b/lb.

33. Mitchell, B., SHOCK ABSORPTION WITH ONE SLOT TUBES, Lockheed Report LR 16369, June 1963.

The work covered in this report is a follow-on of an independent research program that developed a very efficient method of absorbing energy by load control and end roll-up of tubes. This study investigates 12 different materials and improves the control rings to give a smooth, nearly rectangular, energy curve. The load level can be set, by suitable design, at any desired point within the column or local

strength limits of the tube. The maximum efficiency attained here with a 2-inch-diameter by .049 wall 4130 steel tube heat treated to 200,000 psi is 600,000 in.-1b per 1b of tube weight. This represents a mean compressive stress of 189,000 psi over the total 6 inches of travel.

31. Mitchell, B., DESIGN NOTES FOR THE DYNASORB ENERGY ABSORBER, Lockheed Report LR 17201, December 1963.

The Dynasorb Energy Unit, developed in the Lockheed Engineering Laboratory, is adaptable to many applications as a shock absorber or load limiter. Design procedures based on previously reported data are described, and several typical installations are illustrated.

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This report is submitted in fulfillment of the reporting requirements of a 1963 Independent Development Project, "Energy Absorption Products."

35. Mitchell, B., ENERGY ABSORPTION AT HIGH SPEED VERTICAL LANDING, Lockheed Report LR 21023, November 1967.

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This report describes the use of the Lockheed Dynasorb energy absorber in landing impact tests of a 3400-1b simulated vehicle structure. The structure, released from a helicopter and tethered by parachute, impacted at a velocity of 112 feet per second.

36. Perrone, N., RESPONSE OF RATE SENSITIVE FRAMES TO IMPULSIVE LOAD, ASME Journal of Applied Mechanics, February 1971, pp. 49-62.

A relatively convenient method is presented herein to determine the plastic response of rate-sensitive frameworks. The technique represents a significant extension of usual limit analysis type approaches as well as similar efforts applied to rate insensitive structures.

The method consists of assuming a modal deformation pattern for the structure in question, determination of the initial modal velocity utilizing the appropriate criteria, and integration in some form of the equations of motion after estimating the magnitude of the dynamic yield moments at the rate-sensitive plastic hinge locations. It is recommended that an effective hinge length of the order of three beam depths should suffice in estimating the strain rate magnitude. For the perfectly plastic case, the dynamic yield moment is assumed to be a constant with time.

A square portal frame made of a solid rectangular cross-section and loaded under a horizontal impulsive load is considered in some detail. It is shown that a critical initial velocity exists.

The effects of strain hardening and pulse load application are considered. Minor modifications of the impulsively loaded, perfectly plastic situation are necessary to accommodate these extensions.

The usual limitations should be noted. It is assumed that buckling will not occur, that the median surface or membrane effects are negligible, and that elastic effects are negligibly small relative to plastic flow because the former are omitted.

Portal frame experiments under high intensity loading similar to those conducted at Brown University on cantilever beams would be most welcome. Most of the experiments performed to date have been in a much lower load range where elastic and plastic effects are of comparable order. The true limiting strength of the framework could be more completely assessed only if larger loads are applied. Other tests have been reported, and hopefully these results will soon be available.

37. Jones, N., INFLUENCE OF STRAIN-HARDENING AND STRAIN-RATE SENSITIVITY ON THE PERMANENT DEFORMATION OF IMPULSIVELY LOADED RIGID PLASTIC BEAMS, International Journal of Mechanical Science, 1967, Vol. 9, pp. 777-796.

A simple method is presented for estimating the combined influence of strain-hardening and strain-rate sensitivity on the permanent deformation of rigid-plastic structures loaded dynamically. A study is made of the particular case of a beam supported at the ends by immovable frictionless pins and loaded with a uniform impulse. The results of this work indicate that, when stress-hardening or strain-rate sensitivity are considered, permanent deformations are experienced which are similar to those predicted by an analysis retaining both effects simultaneously.

38. Ni, C. M., IMPACT RESPONSE OF CURVED BOX BEAM-COLUMNS WITH LARGE GLOBAL AND LOCAL DEFORMATIONS, General Motors Research Laboratory, Warren, Michigan, AIAA Paper 73-401.

A numerical approach based on a lumped-mass model is developed for investigating the impact response of curved box beam-columns with large global and local deformations. An empirical formula which relates the changes of depth and bending angle of a beam cross-section is obtained to take into account the local deformations of the cross-sections. The strain-hardening and the strain-rate properties of the material are considered in this analysis. The correlation between the present analysis and test results is very good. The results obtained indicate that the strain-wave propagations due to the impact and the strain-rate sensitivity of the material play the key roles in increasing the energy-absorbing capacity of the structure when subjected to high-speed impact.

39. O'Bryan, T.C., and Hatch, H.G., Jr., LIMITED INVESTIGATION OF CRUSH-ABLE STRUCTURES FOR ACCELERATION PROTECTION OF OCCUPANTS OF VEHICLES AT LOW IMPACT SPEEDS, NASA Technical Note D-158, 1958.

A limited investigation is made to determine the characteristics of three materials to see how they can be applied for human protection against accelerations encountered at low impact speeds. As a result, if given man's physiological tolerance to abrupt acceleration, which has not yet been well-defined, an alleviation system can be designed.

Foamed plastics require considerable depth to provide a given stopping distance for impact alleviation, and their use will require some control of rebound. They can be made soft enough to obtain the low onset of acceleration that may be necessary for man where depth is not limited.

Aluminum honeycomb is an efficient material for impact load alleviation from the standpoint of usable material depth, and it exhibits very little rebound. The stiffness of the material results in a very high initial onset rate of acceleration. For many installations this may be controlled by reducing the initial loading area of contact to get the material to start failing.

40. Jones, Norman, THE INFLUENCE OF LARGE DEFLECTION ON THE BEHAVIOR OF RIGID-PLASTIC CYLINDRICAL SHELLS LOADED IMPULSIVELY, <u>Journal of Applied Mechanics</u>, ASME, June 1970, pp. 417-425.

In order to gain some insight into the importance and influence of finite deflections on the response of shells loaded dynamically, this article studies theoretically the behavior of a cylindrical shell subjected to a uniform axisymmetric impulsive pressure. The cylindrical shell is assumed to be made from a rigid, perfectly plastic material, and the external energy imparted to the shell is much greater than the total strain-energy which can be absorbed elastically. The results of the investigation indicate that geometry changes are important and should be retained when studying the behavior of cylindrical shells loaded dynamically.

41. D'Amato, R., STATIC POST-FAILURE STRUCTURAL CHARACTERISTICS OF MULTI-WEB BEAMS, WADC TR 59-112, February 1959.

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The structural behavior of aircraft components is discussed in relationship to the general lethality problem, the concept of postfailure restoring force for built-up structures of multiweb beams is considered, and a relatively simple analysis is developed to compute the static post-failure behavior of multiweb beams in pure bending. An experimental investigation is conducted to examine the validity of the analysis and, within the range of parameters considered, agreement between experiment and theory is satisfactory. Both the theoretical analysis and the experimental results indicated that an arbitrary extrapolation of post-failure data on the basis of ultimate strength can be very misleading.

42. Rawlings, B. ENERGY ABSORPTION OF DYNAMICALLY AND STATICALLY TESTED MILD STEEL BEAMS UNDER CONDITIONS OF GROSS DEFORMATION, International Journal of Mechanical Science, Pergamon Press, Ltd., 1967, Vol. 9, pp. 633-649. Printed in Great Britain.

An account is given of dynamic and static tests on mild steel members deformed under conditions of gross geometry change. An evaluation is made of the rigid-plastic theory, taking account of change of geometry, and also the elastic-plastic theory, assuming deformation to occur in the static mode, in predicting the behavior of the members.

43. Thompson, J.E., VEHICLE CRUSH PREDICTION USING FINITE-ELEMENT TECH-NIQUES, Chrysler Corp., <u>SAE Paper 73-157</u>, January 1973.

The principal objective of this investigation is to develop analytical tools in the form of computer programs which will permit the automobile designer to predict the crush characteristics of a given car structure due to forces generated in a variety of impact modes.

The predictive or control capability is embodied in two large computer programs. "TELSAP" forms, reduces, and inverts the vehicle structure mass matrix expressed relative to a datum coordinate system and writes the mass matrix and its inversion onto a file for reading by the "CRUSH" program. "CRUSH" is a general matrix structural analysis program which calculates the large, plastic, rate-sensitive response of an interconnected beam structure due to known dynamic boundary displacement inputs. The theoretical bases and assumptions employed in developing these programs are described along with a detailed discussion of how the automobile design engineer might use them to develop a new vehicle structure.

Experimental correlation with the computer models is given for a vehicle-to-vehicle 90-degree intersection collision between a special rigid moving barrier and an intermediate-size four-door sedan. The structural model is further correlated with a laboratory test of a clamped-clamped beam struck at its center by an impact pendulum. The correlation indicates general agreement between experimental and analytical results.

By using the computer programs developed in this investigation, the automobile designer is able to reduce the amount of testing required to prove his design, and is able to identify the benefits of a particular structural reinforcement with a minimum of development time and expense.

44. Burgmann, J. B., and Rawlings, B., DYNAMIC PLASTIC ANALYSIS OF PIN-JOINTED FRAMES, <u>International Journal of Mechanical Science</u>, Pergamon Press, 1968, Vol. 10, pp. 967-980, Printed in Great Britain. (Revised 30 July 1968). The paper presents an analysis of a pin-jointed frame subjected to dynamic or impulsive overload, of sufficient magnitude to cause permanent deformation. Rigid-plastic behavior of tensile members is assumed and, as a first approximation, a similar behavior of compressive members is assumed, although modifications to account for other characteristics are also discussed.

The behavior is considered in terms of the kinematic conditions, dynamic conditions, and the load-deformation characteristics assumed for the members.

45. Martin, J.B., MODE APPROXIMATION FOR IMPULSIVELY LOADED STRUCTURES IN THE INELASTIC RANGE, Proceedings of the Southampton 1969 Civil Engineering Material Conference.

A convergence approximation technique, based on the uniqueness proof, is reviewed for impulsively loaded rigid plastic and rigid viscoplastic structures. Emphasis is given to the use of mode or quasimode solutions and their usefulness in establishing a general approximating procedure and in providing insight into the important aspects of the gross structural response.

46. Jensen, W.R., Flaby, W.E., and Prince, N., MATRIX ANALYSIS METHODS FOR ANISOTROPIC INELASTIC STRUCTURES, AFFDL-TR-65-220, April 1966.

Most aerospace structural materials exhibit some degree of anisotropic strain-hardening. During the past few years, several methods have appeared in the literature for introducing inelastic isotropic material behavior effects into existing matrix analysis procedures using the incremental theory of plasticity. A review is presented of these methods and a step-by-step routine known as the "Constant Strain" method is selected for the development of an anisostropic inelastic procedure.

47. Isaakson, G., Armen, H., Jr., and Pipko, A., DISCRETE ELEMENT METHODS FOR THE PLASTIC ANALYSIS OF STRUCTURES, NASA CR 803, October 1967.

This study deals with the extension of finite-element methods to provide analytical means for determining the failure loads of aeronautical structures. Two areas are considered as related to predicting failure loads: inelastic stress analysis in the presence of load cycling, and plastic buckling of the bifurcation type.

Finite-element inelastic stress analysis methods are extended to take into account the Bauschinger effect for biaxial stress states using a plasticity theory based on Ziegler's modification to Prager's kinematic hardening theory. This methodology is applied to several structures representative of aeronautical construction, including a notched plate, a shear lag specimen, and a swept wing. Good correlation is obtained between analytical and experimental results for

the strains at the root of the notched plate subjected to load cycling in the plastic range.

Finite-element buckling methods are also extended to consider plastic buckling using Stowell's formulation for implementing a deformation plasticity theory into the buckling theory. Sample calculations are carried out for the plastic buckling of a flat plate with various geometries and edge conditions.

48. Stricklin, J.A., et al, NONLINEAR DYNAMIC ANALYSIS OF SHELLS OF REVOLUTION BY MATRIX DISPLACEMENT METHOD, <u>AIAA Journal</u>, Vol. 9, No. 4, April 1971, p. 629.

A formulation and computer program is developed for the geometrically nonlinear dynamic analysis of shells of revolution under symmetric loads. The nonlinear strain energy expression is evaluated using linear functions for all displacements. Five different procedures are examined for solving the equations of equilibrium, with Houbolt's method proving to be the most suitable. Solutions are presented for the symmetrical and asymmetrical buckling of shallow caps under step pressure loadings and a wide variety of other problems, including some highly nonlinear ones.

49. Owens, R. H., and Symonds, P.S., PLASTIC DEFORMATIONS OF A FREE RING UNDER CONCENTRATED DYNAMIC LOADING, ASME Journal of Applied Mechanics, December 1955, p. 524.

A concentrated time-dependent force acts on an unsupported thin ring along a diameter. The problem considered in this paper is to determine the deformations of the ring when the force magnitudes are such that plastic strains occur which are large compared to the elastic strains. By neglecting elastic strains and assuming ideally plastic behavior, approximations to the final deformations of the ring are obtained. The analysis is developed for force pulses of arbitrary shape, but numerical results are obtained only in the special case of a rectangular force pulse. A criterion is stated for conditions when this type of analysis can be expected to provide satisfactory results.

50. Lee, H., and Symonds, P.S., LARGE PLASTIC DEFORMATIONS OF BEAM UNDER TRANSVERSE IMPACT, ASME Journal of Applied Mechanics, September 1952, p. 308.

A comparatively simple method of analysis is developed to determine the deformations in a beam subjected to lateral impact of such a magnitude that plastic strains occur which are large compared with elastic strains. A useful approximation to the motion then can be obtained by neglecting elastic strains and considering rigid-body motion of segments of the beam joined at plastic hinges where the

entire deformation takes place. A method of analyzing such a situation is described and applied to a beam subjected to central impact. The approximate final permanent deformation is obtained; this includes deformation during application of the load, and plastic flow which continues afterward when the kinetic energy of the motion generated by the impact is transformed into additional plastic deformation. A criterion is given for conditions when this type of theory can be expected to provide a satisfactory analysis. The method of solution provides an interesting analogy to the concept of static determinacy which has been used in the analysis of quasi-static plastic-flow problems.

51. Prager, W., A NEW METHOD OF ANALYZING STRESSES AND STRAINS IN WORK-HARDENING PLASTIC SOLIDS, <u>ASME Journal of Applied Mechanics</u>, December 1956, p. 493.

For work-hardening plastic solids, segment-wise linear yield conditions and the associated flow rules constitute a reasonable compromise between the mathematically convenient but physically unsound total stress-strain laws and the physically sound but mathematically inconvenient incremental laws. They allow total stress-strain laws to be used in the small, but retain the characteristic features of the incremental laws in the large. The use of a segmentwise linear yield condition and the associated flow rule is illustrated by the analysis of the bending moments and deflections of a simply supported circular plate that is made of a work-hardening material and subjected to a uniformly distributed transverse load.

52. Morino, L., Leech, J.W., and Witmer, E.A., AN IMPROVED NUMERICAL CAL-CULATION TECHNIQUE FOR LARGE ELASTIC-PLASTIC TRANSIENT DEFORMATIONS OF THIN SHELLS, Part 1, <u>ASME Journal of Applied Mechanics</u>, June 1971, p. 423.

In this paper, the governing differential relations which describe the large-deflection elastic-plastic dynamic responses of arbitrarily shaped thin Kirchhoff shells are given, including recent improvements. These relations are then cast into finite-difference form for numerical solutions. These finite-difference relations are employed in a computer program, PETROS 2, which has been applied to evaluate the "analysis improvements" by comparing PETROS 2 predictions with those of the earlier analysis and with experimental results.

53. Pifko, A. Issakson, A FINITE-ELEMENT METHOD FOR THE PLASTIC BUCKLING ANALYSIS OF PLATES, Grumman Aerospace Corporation, Bethpage, N.Y., AIAA Journal of Applied Mechanics, Vol. 7, No. 10, October 1969.

An existing finite-element technique for elastic buckling of plates is extended to include the case of plastic buckling. The Stowell theory for the plastic buckling of flat plates is used in conjunction with the finite-element technique. Application is made to rectangular

plates, and results are presented for a variety of boundary support conditions and several different edge loading conditions.

54. Armen, H., Jr., Pifko, A., and Levine, H.S., FINITE ELEMENT ANALYSIS OF STRUCTURES IN THE PLASTIC RANGE, NASA CR 1649, February 1971.

The present report is concerned with the development of finiteelement methods for the treatment of the nonlinear behavior of complex structures. It represents an extension of a previous study reported in NASA Contractor's Report CR-803. The nonlinearity may be of two types - material nonlinearity associated with plastic deformation, and geometric nonlinearity associated with the changing geometry of the structure as it deforms - or it may involve a combination of the two. Effects due to creep and other time-dependent material properties are neglected.

The methods developed are applicable to loading conditions that cause membrane stress states or pure bending, or both in combination. The Prager-Ziegler kinematic hardening theory of plasticity is incorporated in the finite-element methods to allow for consideration of the plastic response of structures subjected to realistic loading conditions, including cyclic loadings that cause stress reversals into the plastic range. Ideally, plastic behavior is also included to provide capability for predicting the collapse load of structures. The plasticity theory is implemented in the finite-element analysis by using an incremental approach in conjunction with the initial strain concept, with plastic strains interpreted as initial strains.

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The treatment of geometric nonlinearity requires use of an incremental technique in which the internal forces and configuration of the structure are continuously updated to account for its changing geometry.

The methods developed are applied to a number of sample structures. For membrane stress states alone, the analysis employs a triangular finite element in which stress and strain vary linearly. This element is used for the plastic analysis of a variety of structures characterized by regions of rapid stress variation and subjected to cyclic loading resulting in reversed plasticity. Comparisons of the results of the analysis and experimental data indicate good correlation.

Plastic analyses are also performed for a variety of beam and plate structures. These problems make use of refined rectangular and triangular finite elements. Among the problems considered are rectangular, circular, and annular plates with various boundary conditions. Once again, comparisons with results of other available analyses are favorable.

Problems of combined bending and stretching of plates are also considered. Results are obtained for rectangular and circular plates. Results for combined geometric and material nonlinearity are presented for beams and arches.

55. Mallett, R. H., AUTOMATED METHOD FOR THE LARGE DEFLECTION AND IN-STABILITY ANALYSIS OF THREE-DIMENSIONAL TRUSS AND FRAME ASSEMBLIES, AFFDL-TR-66-102, December 1966.

The computer program presented in this report was developed to predict large deflection behavior of three-dimensional truss and frame assemblies. The solutions are obtained by the direct minimization of the total potential energy with respect to the displacement variables rather than by solving nonlinear matrix equations. Sample problems are presented to demonstrate the analysis capability developed. Instructions for the preparation of the input data and the FORTRAN IV source program listing are included.

56. Przemieniecki, J.S., MATRIX METHODS IN STRUCTURAL MECHANICS, AFFDLTR-66-80, Conference held October 26-28, 1965.

The Conference on Matrix Methods in Structural Mechanics held at Wright-Patterson Air Force Base on 26-28 October 1965 was sponsored jointly by the Air Force Flight Dynamics Laboratory, Research and Technology Division, Air Force Systems Command, and the Air Force Institute of Technology, Air University. The purpose of the conference was to discuss the recent developments in the field of matrix methods of structural analysis and design of aerospace vehicles.

57. Berke, L., PROCEEDINGS OF THE SECOND CONFERENCE ON MATRIX METHODS IN STRUCTURAL MECHANICS, AFFDL-TR-68-150, Conference held October 15-17, 1968.

The Second Conference on Matrix Methods in Structural Mechanics, sponsored by the Air Force Institute of Technology (AFTT), Air University, and the Air Force Flight Dynamics Laboratory (AFFDL), Air Force Systems Command, was held on 15-17 October 1968. The purpose of the conference is to discuss the recent developments in matrix structural analysis and design of structural systems. This volume contains all the papers presented at the conference.

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Forty papers were presented at the conference in seven sessions, entitled Structural Weight Optimization, Dynamics, General Elements, Curved Elements, Applications, General Methods, and Nonlinear Analysis. The papers covered practically all major aspects of recent research and development work on matrix methods in structural mechanics.

58. Symonds, P.S., SURVEY OF TECHNICAL METHODS OF ANALYSIS FOR PLASTIC DEFORMATION OF STRUCTURES UNDER DYNAMIC LOADING, BU/NSRDC/1-67, Brown University, Providence, R.I., 1967.

This survey attempts to make a critical study of methods described in the literature for the analysis of metal structures under dynamic loading to plastic deformation.

Analyses have now appeared in the literature of a considerable variety of structures of engineering interest. They include beams (under many conditions of loading, support, and materials), rings, arches, frames, (simple rectangular bents), plates (circular and rectangular), membranes (i.e., plates with deflections greatly exceeding the thickness), and shells (axially symmetric loading on cylinders, spheres, and spherical caps). Most of these have been obtained by a rigid-plastic type of analysis (in which strain rates are assumed zero unless a yield condition is satisfied). A few have been obtained by wholly numerical approaches of finite-difference type.

Experiments reported in the literature have, in most cases, shown that the actual permanent deflections are smaller than those predicted on the basis of plastic properties determined by quasi-static tests, the predictions often being in error by as much as 100 percent or more for mild steel, with smaller discrepancies for other metals, such as aluminum alloys or high-strength steels. Strengthening under conditions of rapid straining has been considered the principal cause of such discrepancies; when it has been possible to modify the analysis to take account of the increase of yield and flow stresses at high strain rates, much better agreement has, in most cases, been obtained.

59. Semonian, J.W., and Anderson, P.A., AN ANALYSIS OF THE STABILITY AND ULTIMATE BENDING STRENGTH OF MULTIWEB BEAMS WITH FORMED CHANNEL WEBS, NACA Technical Note TN 3232, 1954.

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Design curves and procedures are presented for calculating the stresses for instability and failure of multiweb beams with formed-channel webs. The ultimate bending strength of this type of construction is shown to depend upon the deflectional stiffness of the web attachment flanges. A simple criterion is also given for predicting whether a multiweb beam with a given attachment-flange design will be susceptible to a wrinkling instability or will buckle as if the webs are integrally joined to the cover skins.

The criteria for predicting buckling and failure stresses are compared with experimental data. These criteria are sensitive to the offset, pitch, and diameter of the rivets used on the web attachment flanges, and the riveting specification is, therefore, emphasized as an important design consideration.

60. Semonian, J.W., and Peterson, J.P., AN ANALYSIS OF THE STABILITY AND ULTIMATE COMPRESSIVE STRENGTH OF SHORT SHEET-STRINGER PANELS WITH SPECIAL REFERENCE TO THE INFLUENCE OF THE RIVETED CONNECTION BETWEEN SHEET AND STRINGER, NACA Technical Report TR 1255, 1956.

A method of strength analysis of short sheet-stringer panels subjected to compression is presented which takes into account the effect that the riveted attachments between the plate and the stiffeners have on the strength of panels. An analysis of experimental data shows that panel strength is highly influenced by rivet pitch, diameter, and location and that the degree of influence for a given riveting depends on the panel configuration and panel material.

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TABLE I. LITERATURE MATRIX CATEGORIZATION

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TEST DATA

DYNAMIC TEST INSTALLATION

The dynamic test setup uses existing components. The drop carriage assembly is the same one that wes utilized during the drop test of the fuselage bumper, the results of which are reported in Reference 1. Between the test specimen support and the ground are installed six load cells (two each along the north and south edges of the support and one each along the east and west edge of the support). The load cells are used for tests 7 and 8. In the previous dynamic tests (4, 5, 6), the specimen support is grouted to the concrete slab ground. The installation provides sufficient free-fall clearance (14 ft) to perform 30-ft/sec impact tests. The setup has the flexibility of performing higher impact velocity drop tests by adding additional frames, thus increasing the available free-fall distance. Figure 1 shows a layout of the test installation and notes the various items and assemblies that form a part of the complete installation.

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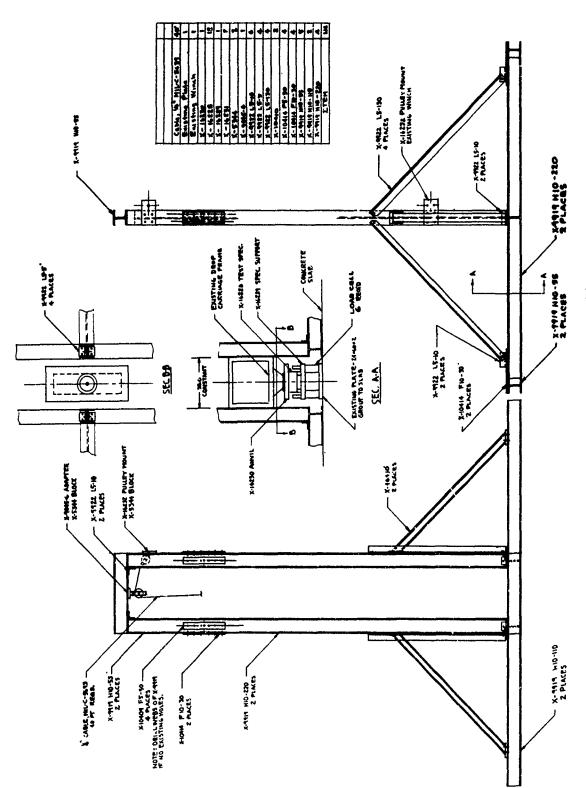
TEST SPECIMENS

A complete description of the test specimen overall dimensions, bulkhead and end beam web thicknesses, number and thickness of angles, design configuration, nominal weight, and type of test performed for each specimen is shown in Volume I, Table XVII. A complete set of drawings for the various design configurations is presented herein as described in Table II below:

Table II.	SPECIMEN DRAWING IDENT	IFICATION
Figure	Applicable Specimen	Drawing X-16628A
2	1	-1
3	2,3,4	- 2
4	5,6	-3
5	7,8,9	-4
6	10,11,12	- 5

RECORDED SCAN (TIME) HISTORIES

A complete set of pertinent recorded test data is presented in Figures 7 through 80. Load-deflection curves for each of the test specimens are presented in Volume I in the section entitled Substructure Test Program.



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Figure 1. Dynamic Test Installation.

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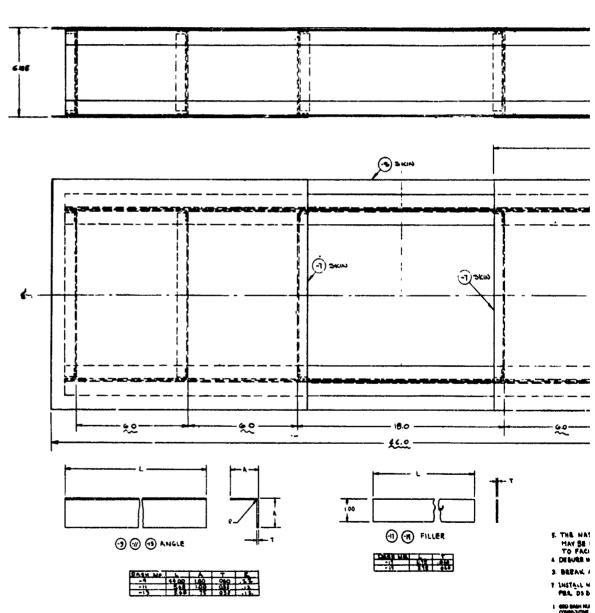
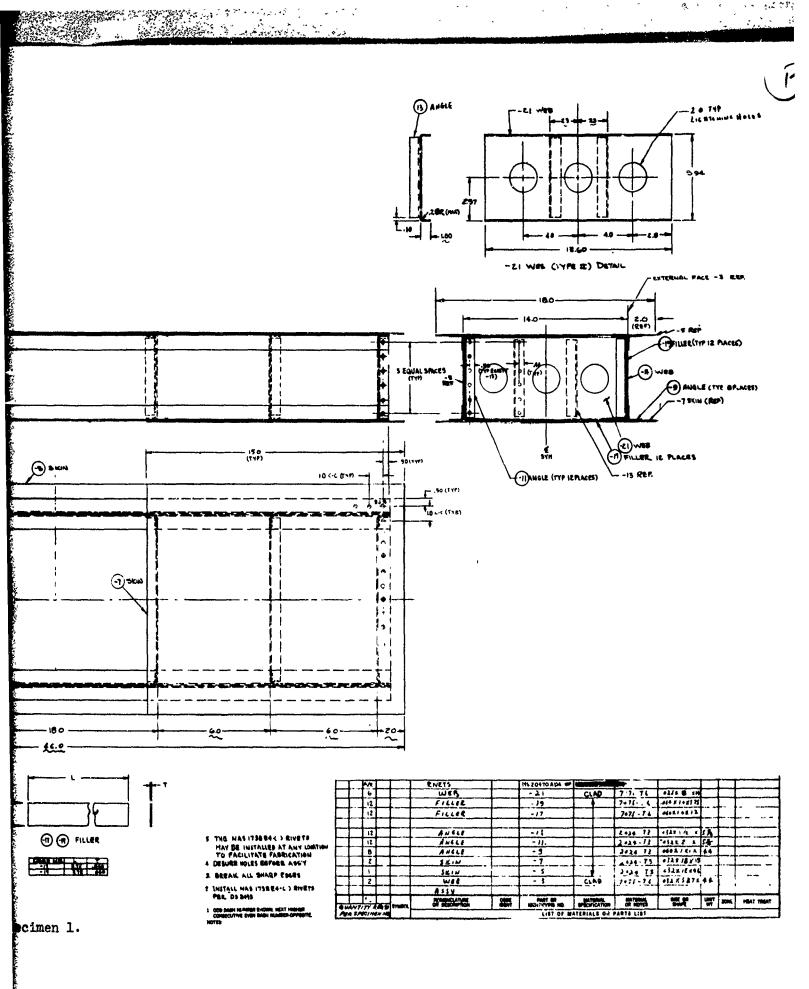


Figure 2. Drawing for Specimen 1.



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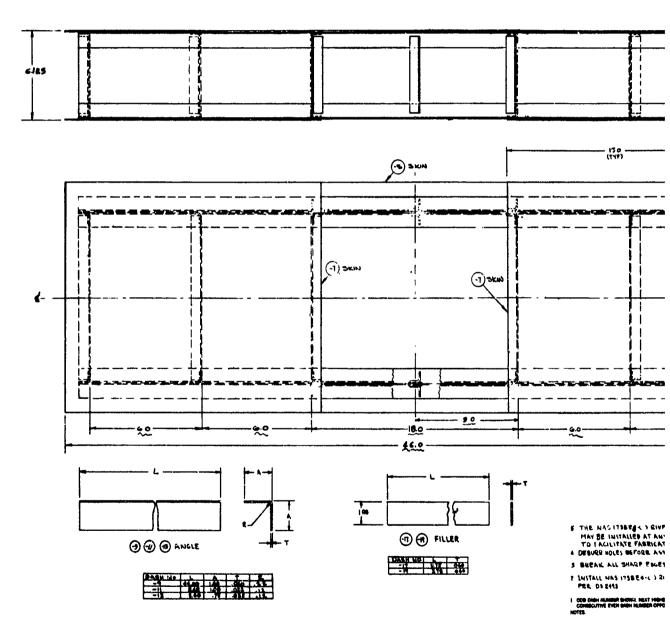
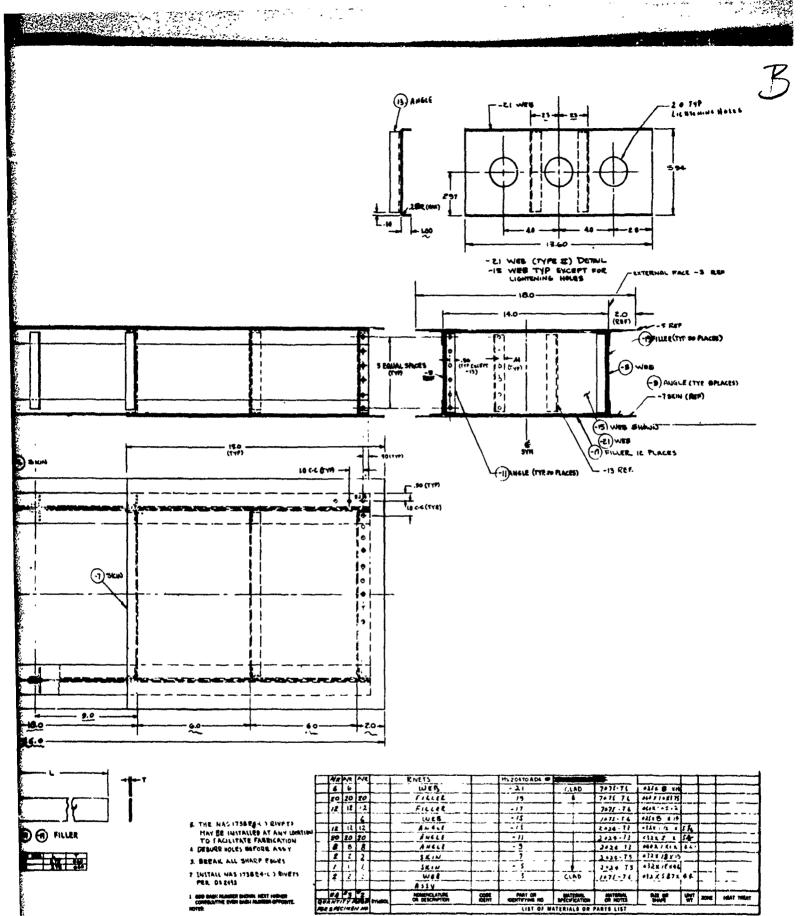


Figure 3. Drawing for Specimens 2, 3, and 4.

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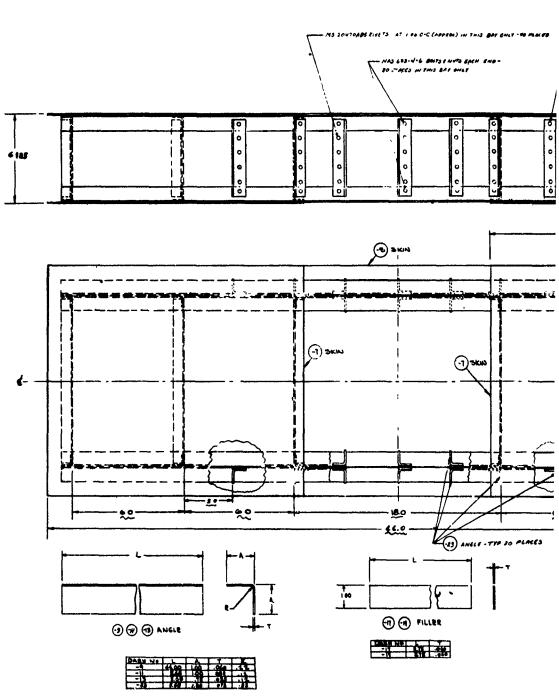
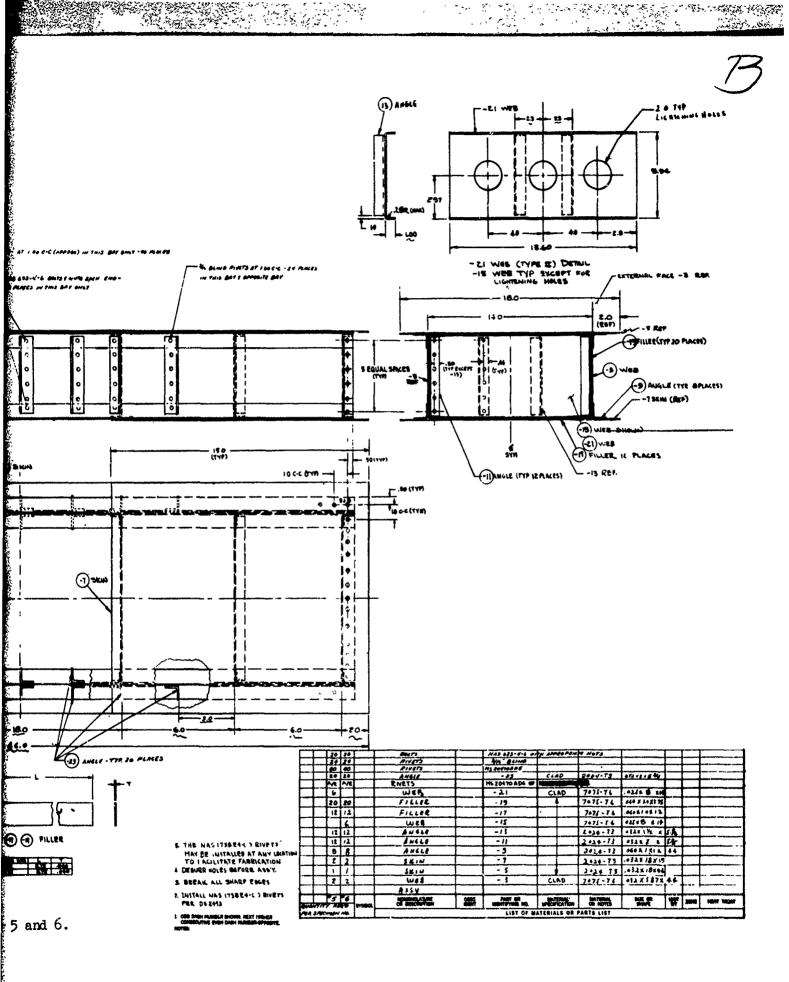
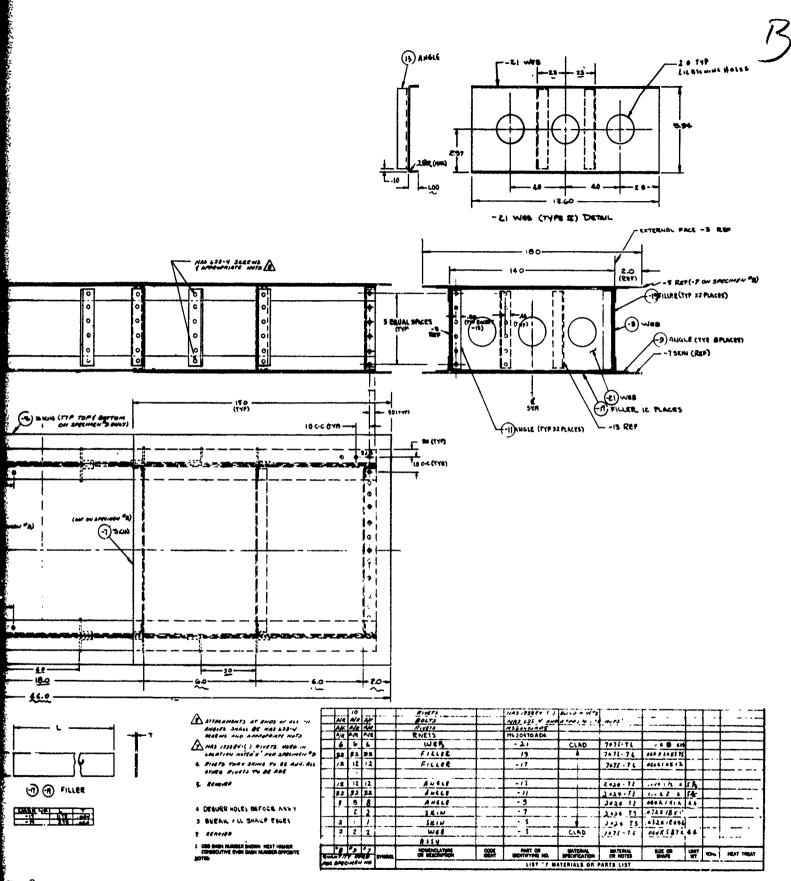


Figure 4. Drawing for Specimens 5 and 6.

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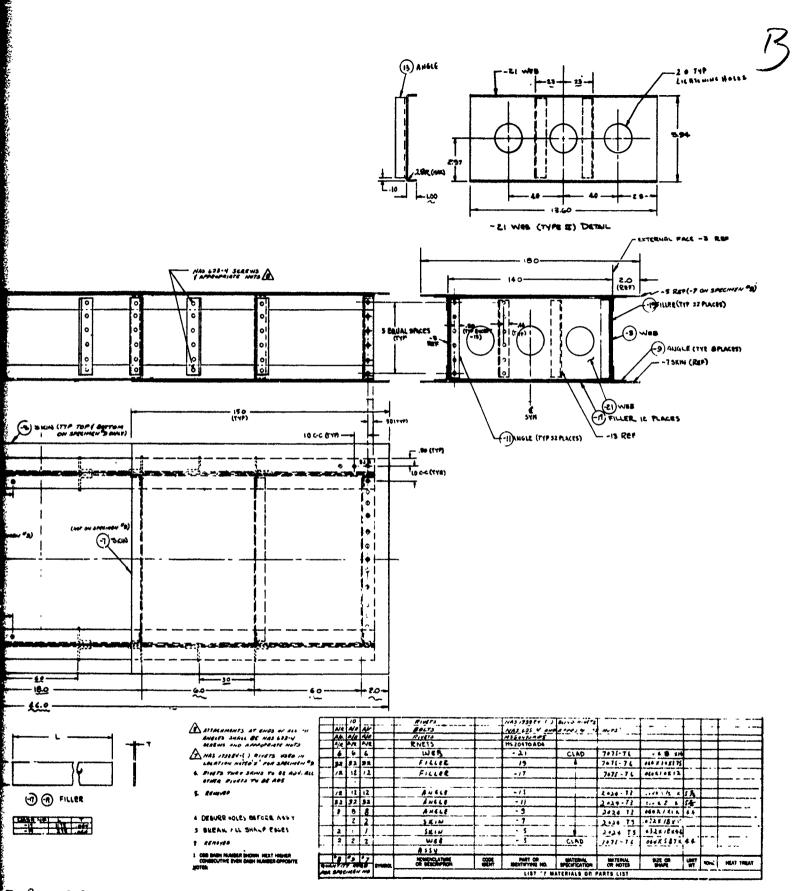




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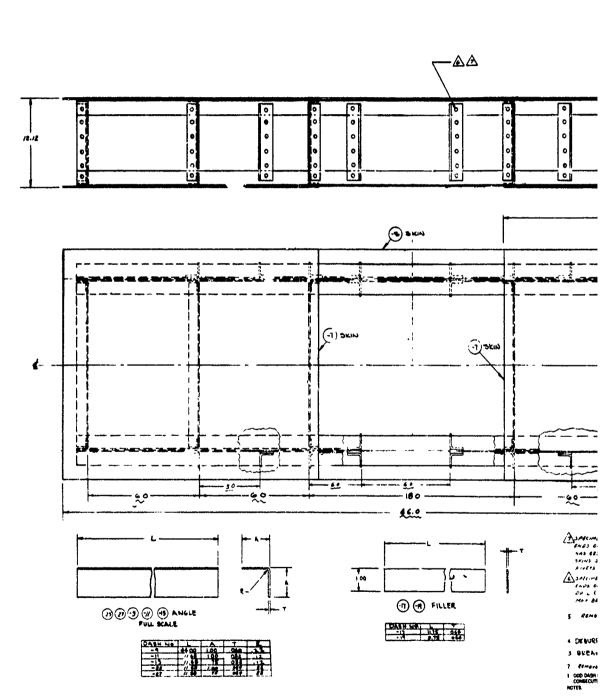
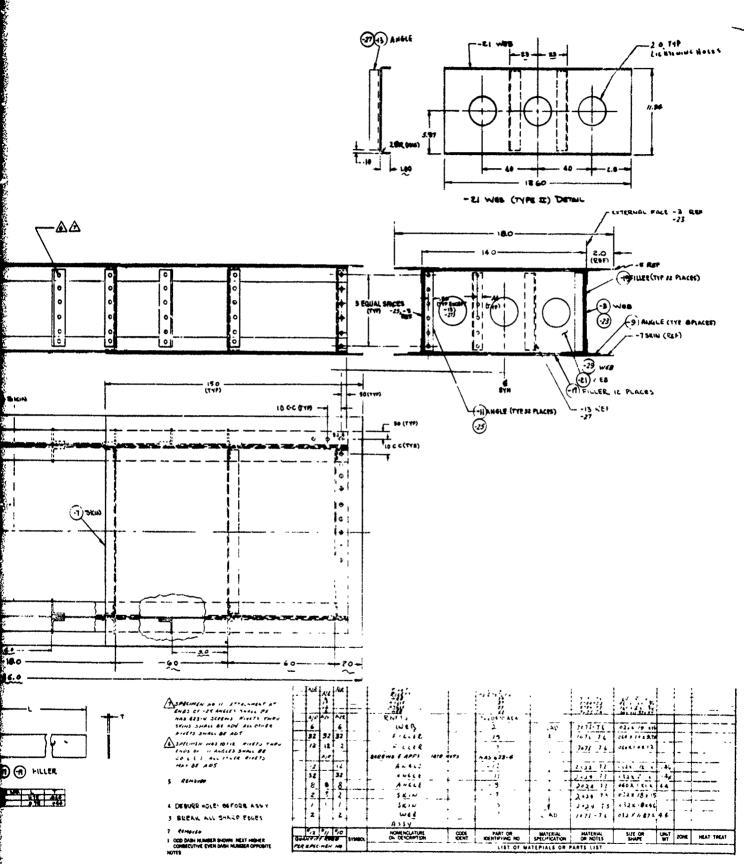


Figure 6. Drawing for Specimens 10, 11, and 12.

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11, and 12.

The data in this section includes load, deflection, acceleration (tests 4-8) and strain versus scan plots. Table III shows the test data available in the section for each specimen.

The sampling rate for test 4 is 750 scans/second. The sampling rate for tests 5 through 8 is 1500 scans/second; thus for the dynamic tests the time in seconds can be obtained by dividing the soscissa scale (scan) by 750 for test 4 and by 1500 for tests 5 through 8. Positive strain is compression for tests 1, 2, 3, 9, 10, 11, 12 and tension for tests 4, 5, 6, 7 and 8.

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			Table	HII.	TEST D	Table III. TEST DATA IDERLIFICATION	MILTEI(CATION				
					Test	Specimen	q					
Date. Item	н	2	3	†7	5	9	2	8	6	10	Ħ	প্ল
Load	×	×	×				×	×	×	×	×	×
Deflection	×	×	×	×	×	×	×	×	×	×	×	×
Strain Gage 1A	×	×	.×	×	×	×	×	×	×	×	×	×
Strain Gage 1B	×	×	×	×	×	×	×	×	×	×	×	×
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Strain Gage 3	×	×	×									
Strain Gage 4	×	×	×									
Strain Gage 5	×	×	×									
Strain Gage 6	×	×	×	×	×	×					-	
Acceleration				×	×	×	×	×				
Applicable Figures	7 thru 13	14 thru 20	21 thru 29	30 thru 38	39 thru 44	45 thru 50	51 thru 60	61 thru 68	69 thru 71	72 thru 74	75 thru 77	78 thru 80

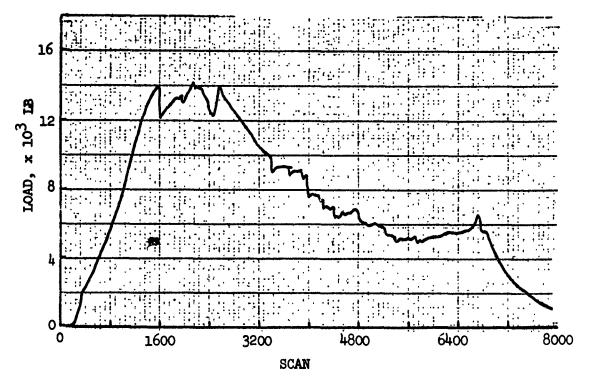


Figure 7. Load Versus Scan, Test Specimen 1.

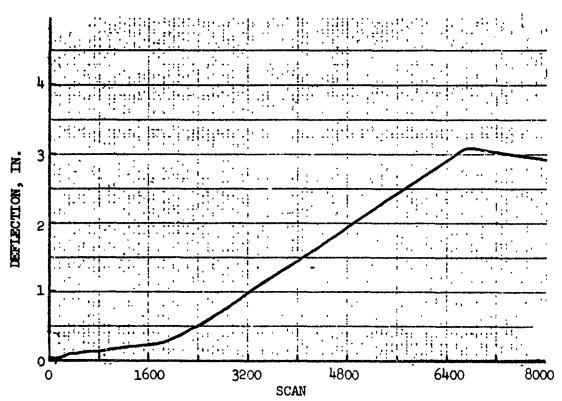


Figure 8. Deflection Versus Scan, Test Specimen 1.

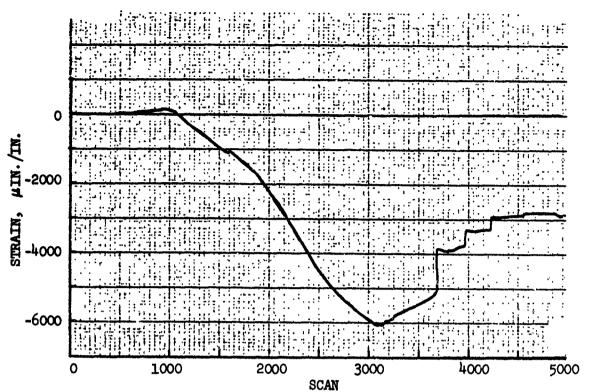


Figure 9. Strain Gage 1A Versus Scan, Test Specimen 1.

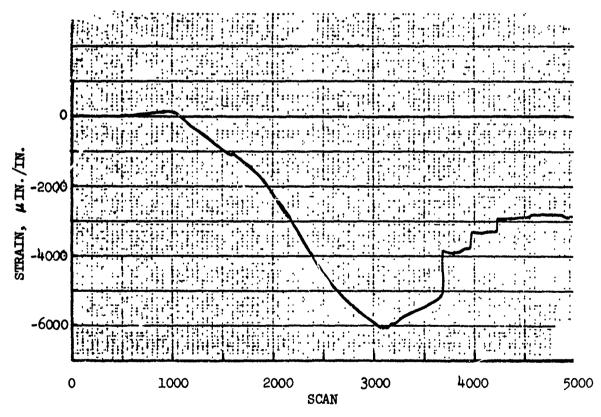


Figure 10. Strain Gage 1B Versus Scan, Test Specimen 1.

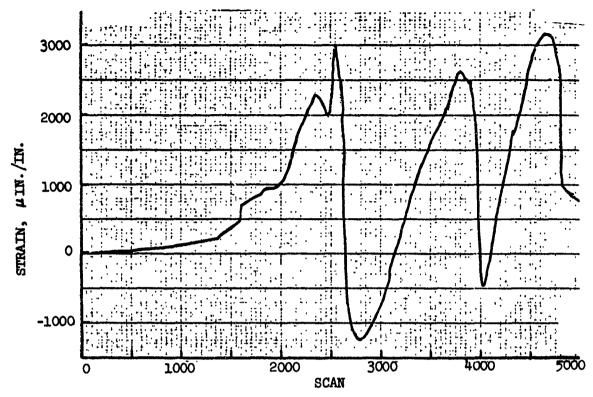


Figure 11. Strain Gage 2 Versus Scan, Test Specimen 1.

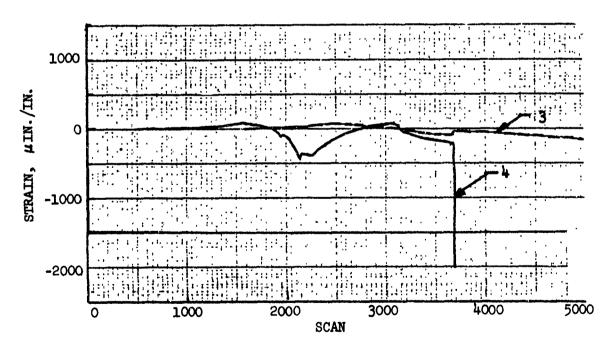
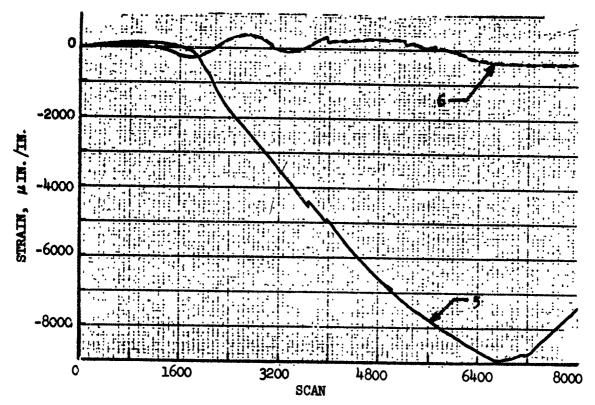


Figure 12. Strain Gages 3 and 4 Versus Scan, Test Specimen 1.



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Figure 13. Strain Gages 5 and 6 Versus Scan, Test Specimen 1.

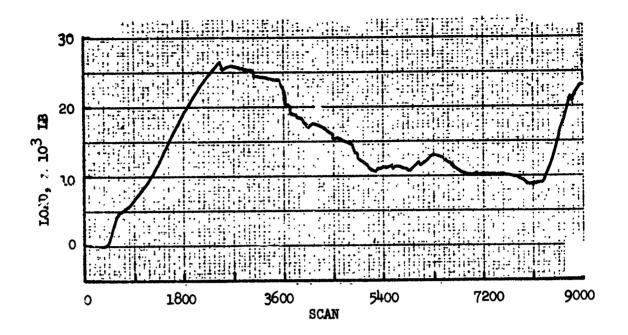


Figure 14. Load Versus Scan, Test Specimen 2.

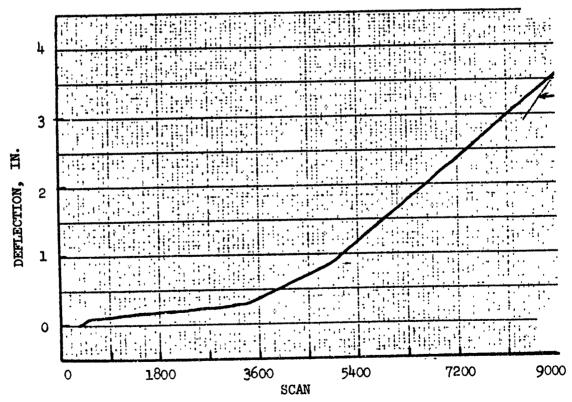


Figure 15. Deflection Versus Scan, Test Specimen 2.

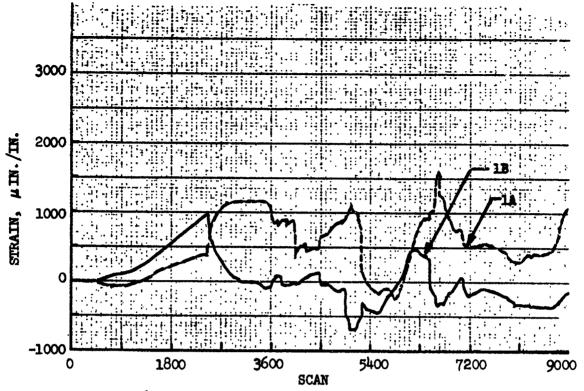


Figure 16. Strain Gages 1A and 1B Versus Scan, Test Specimen 2.

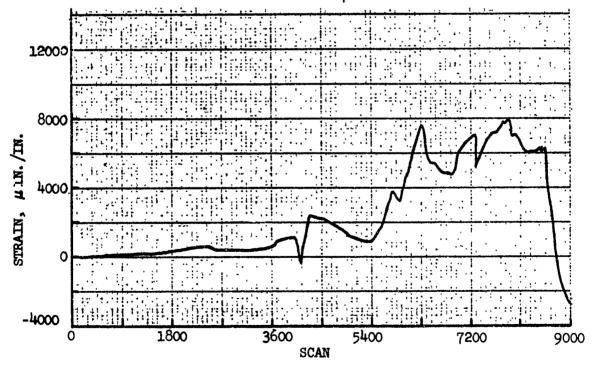


Figure 17. Strain Gage 2 Versus Scan, Test Specimen 2.

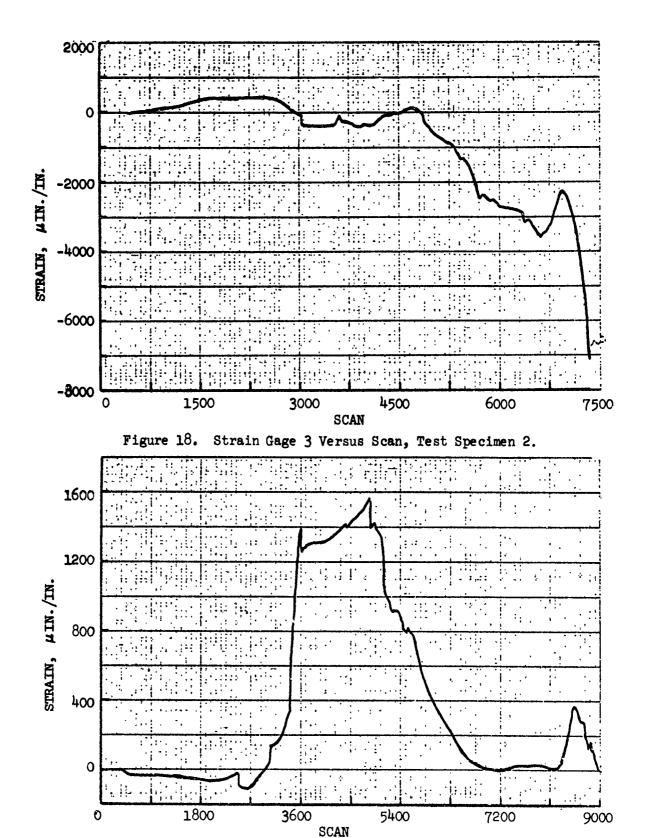


Figure 19. Strain Gage 4 Versus Scan, Test Specimen 2.

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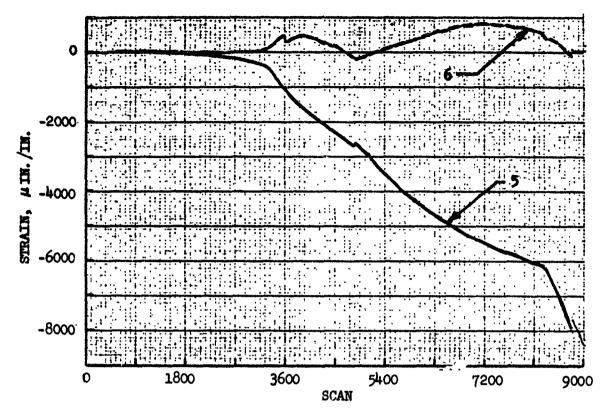


Figure 20. Strain Gages 5 and 6 Versus Scan, Test Specimen 2.

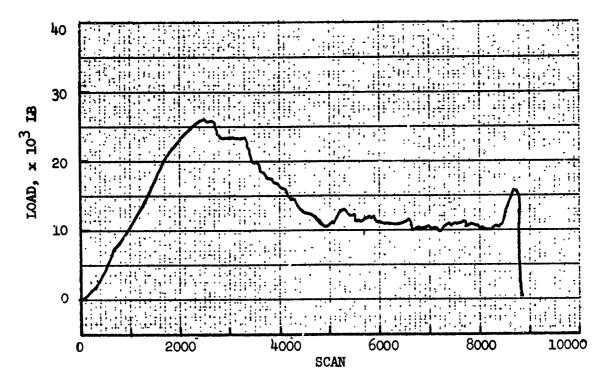


Figure 21. Load Versus Scan, Test Specimen 3.

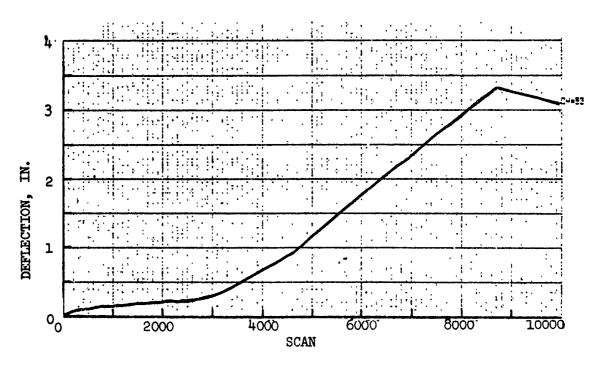


Figure 22. Deflection Versus Scan, Test Specimen 3.

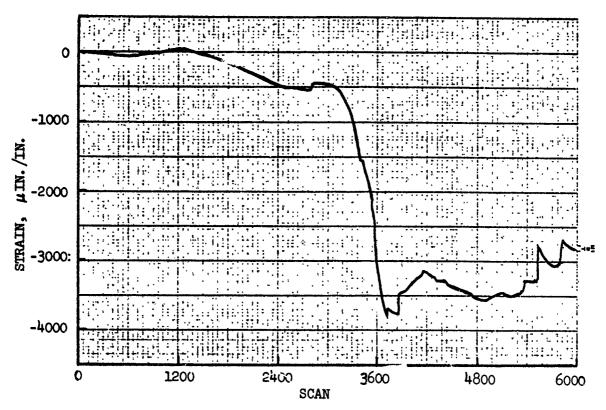


Figure 23. Strain Gage LA Versus Scan, Test Specimen 3.

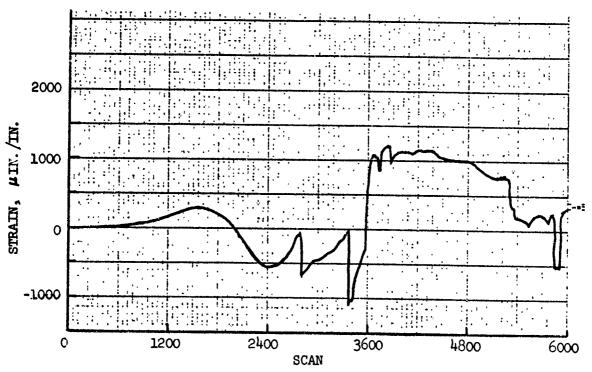


Figure 24. Strain Gage 1B Versus Scan, Test Specimen 3.

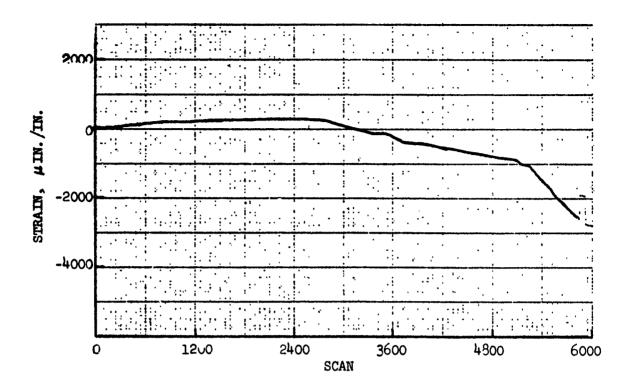


Figure 25. Strain Gage 2 Versus Scan Test Specimen 3.

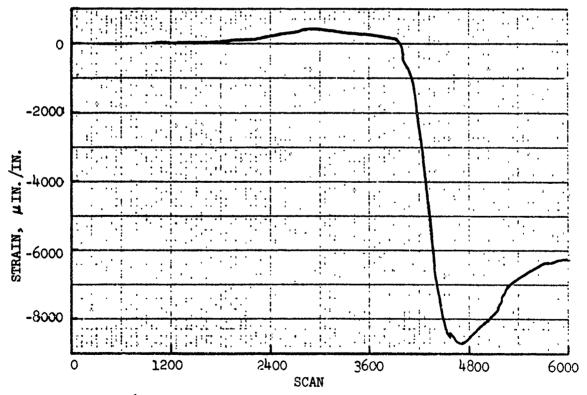


Figure 26. Strain Gage 3 Versus Scan, Test Specimen 3.

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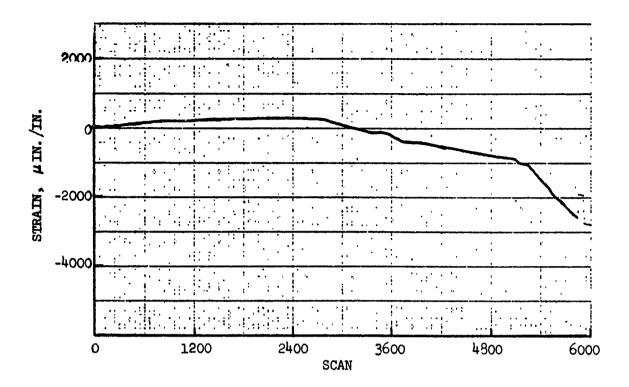


Figure 25. Strain Gage 2 Versus Scan, Test Specimen 3.

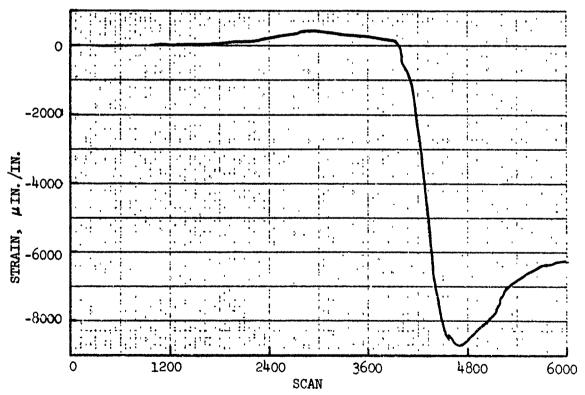


Figure 26. Strain Gage 3 Versus Scan, Test Specimen 3.

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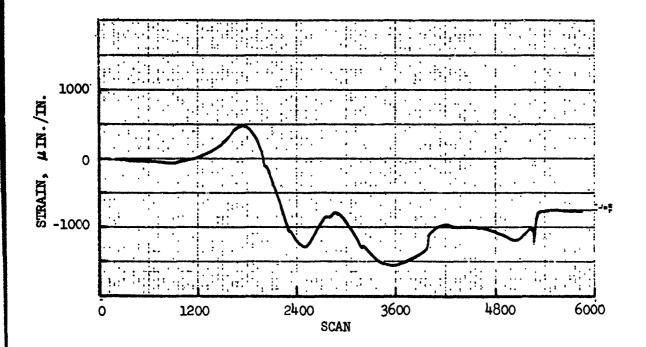


Figure 27. Strain Gage 4 Versus Scan, Test Specimen 3.

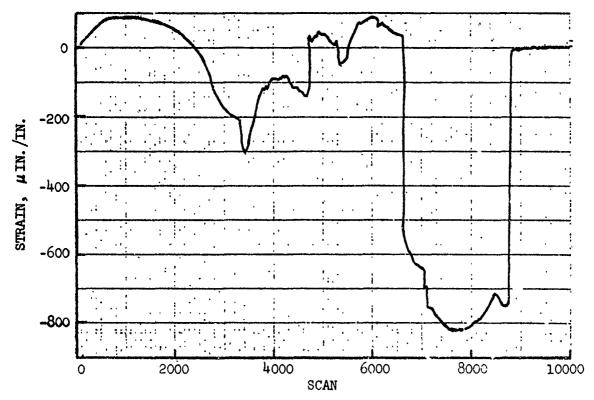


Figure 28. Strain Gage 5 Versus Scan, Test Specimen 3.

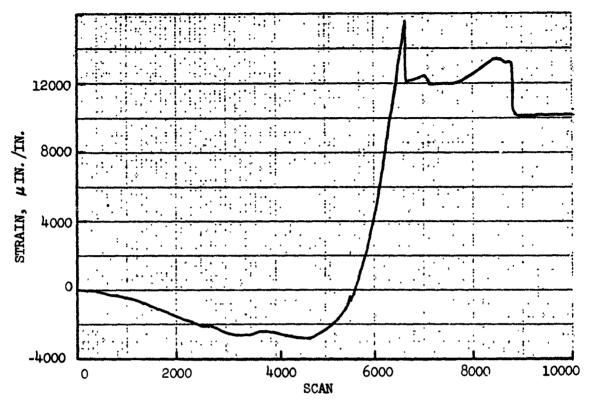


Figure 29. Strain Gage 6 Versus Scan, Test Specimen 3.

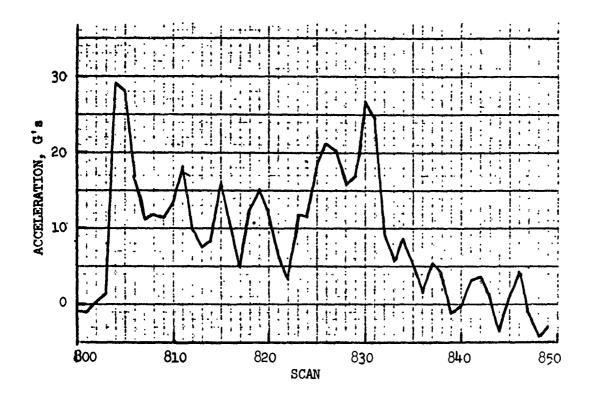


Figure 30. Acceleration Versus Scan, Test Specimen 4.

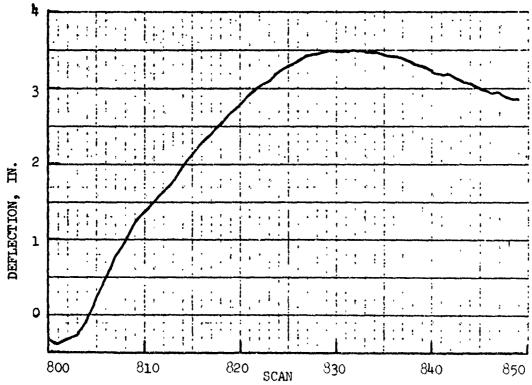


Figure 31. Deflection Versus Scan, Test Specimen 4.

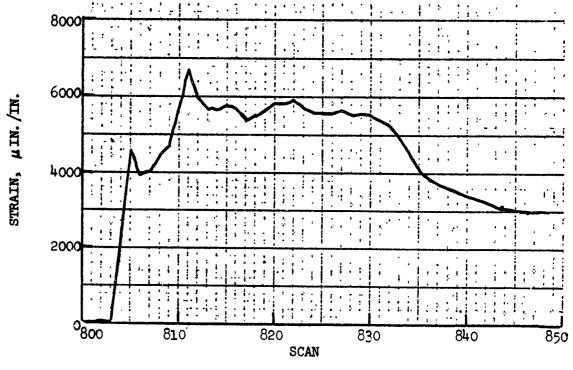


Figure 32. Strain Gage 1A Versus Scan, Test Specimen 4.

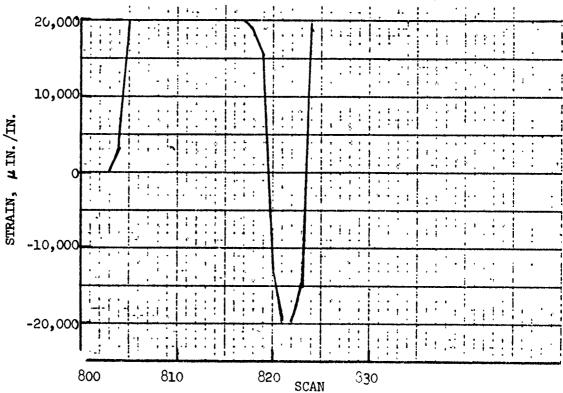


Figure 33. Strain Gage 1B Versus Scan, Test Specimen 4.

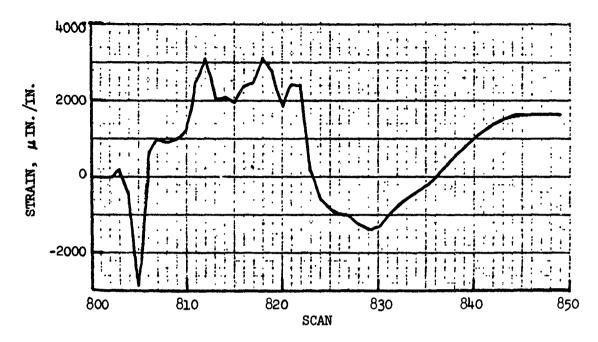


Figure 34. Strain Gage 2 Versus Scan, Test Specimen 4.

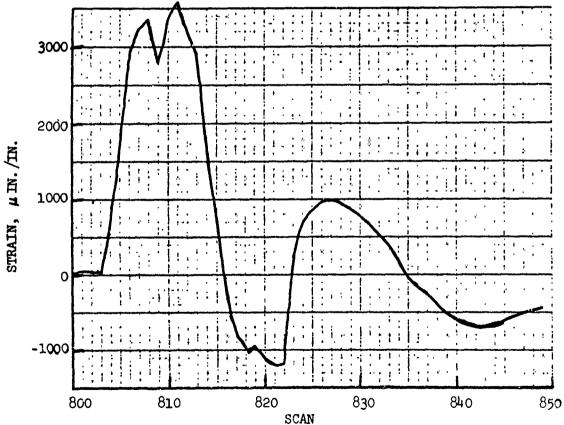


Figure 35. Strain Gage 3 Versus Scan, Test Specimen 4.

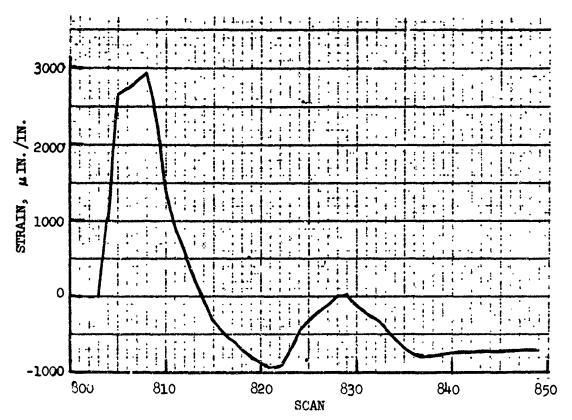


Figure 36. Strain Gage 4 Versus Scan, Test Specimen 4.

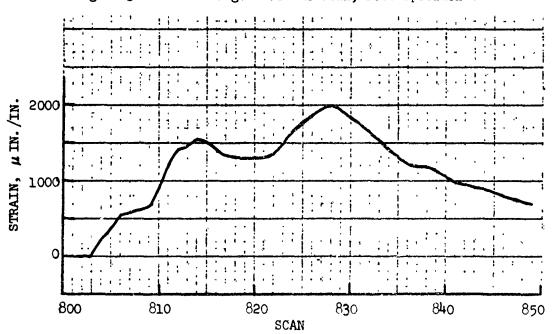


Figure 37. Strain Gage 5 Versus Scan, Test Specimen 4.

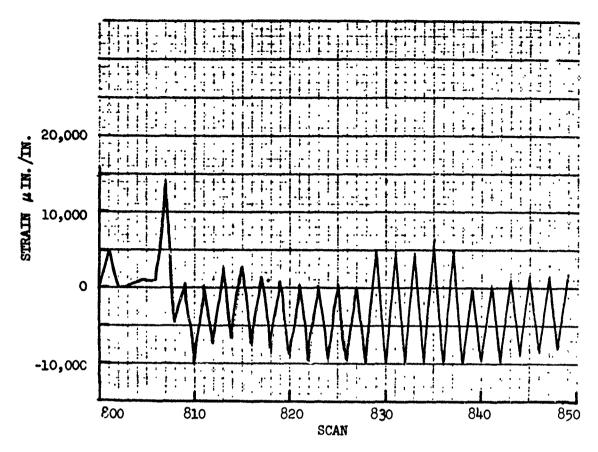


Figure 38. Strain Gage 6 Versus Scan, Test Specimen 4.

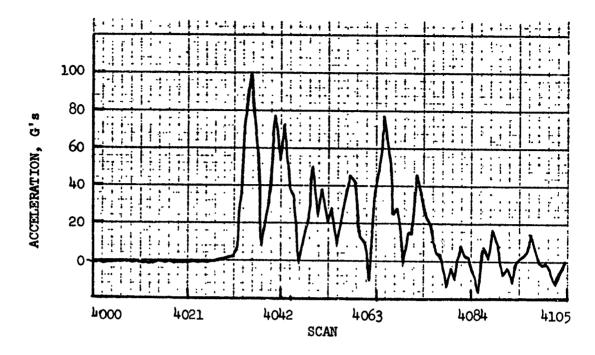


Figure 39. Acceleration Versus Scan, Test Specimen 5.

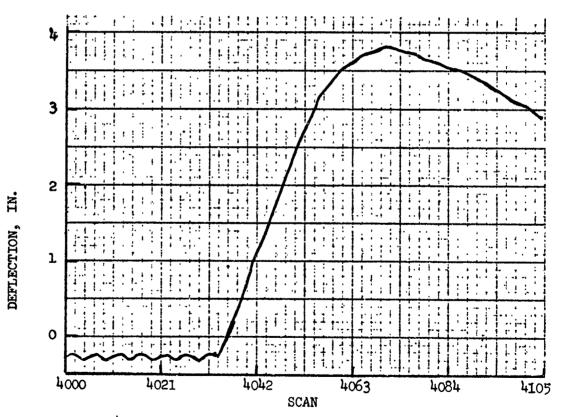


Figure 40. Deflection Versus Scan, Test Specimen 5.

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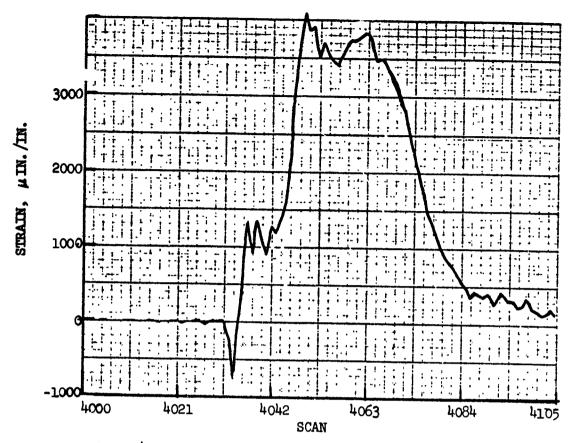


Figure 41. Strain Gage 1A Versus Scan, Test Specimen 5.

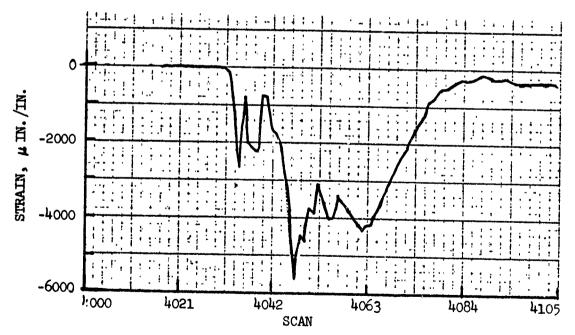


Figure 42. Strain Gage 1B Versus Scan, Test Specimen 5.

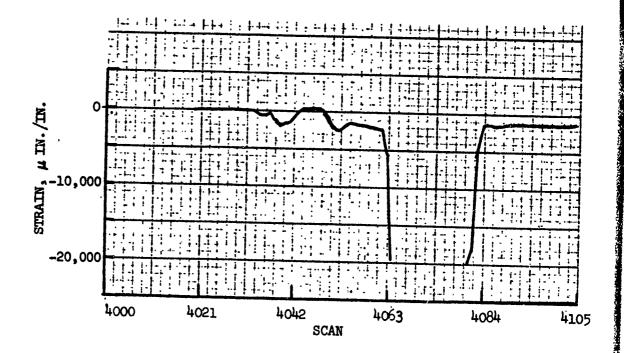


Figure 43. Strain Gage 2 Versus Scan, Test Specimen 5.

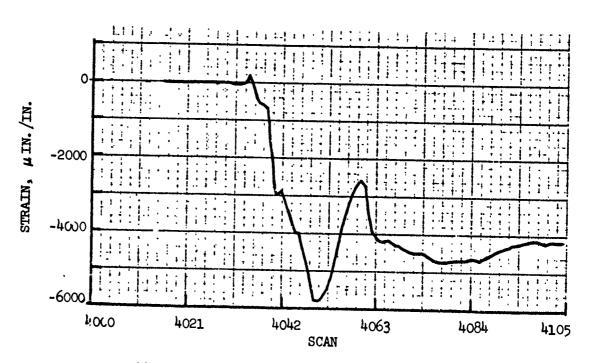


Figure 44. Strain Gage 4 Versus Scan, Test Specimen 5.

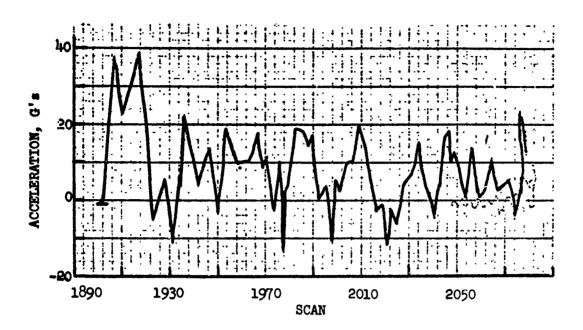


Figure 45. Acceleration Versus Scan, Test Specimen 6.

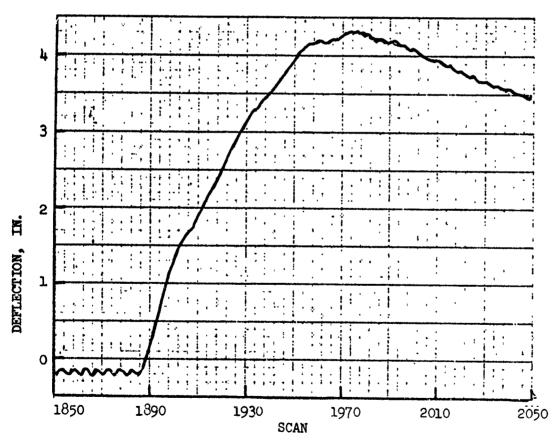


Figure 46. Deflection Versus Scan, Test Specimen 6.

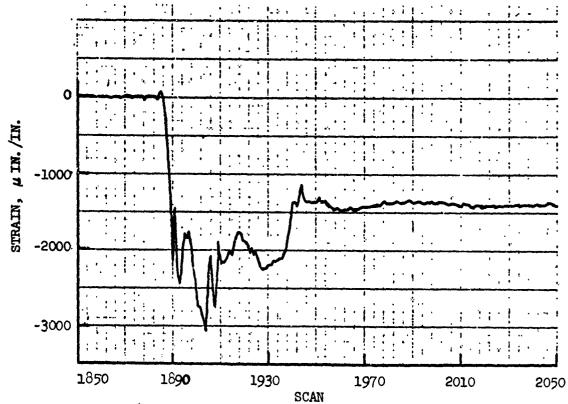
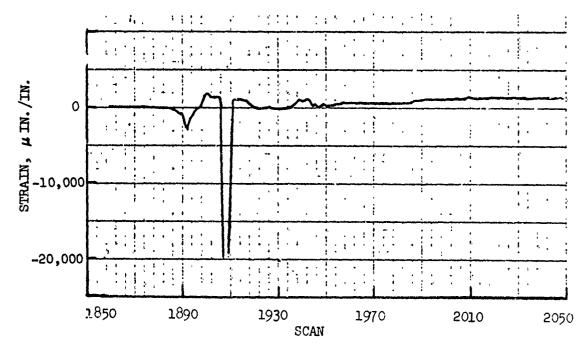


Figure 47. Strain Gage 1A Versus Scan, Test Specimen 6.



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Figure 48. Strain Gage 1B Versus Scan, Test Specimen 6.

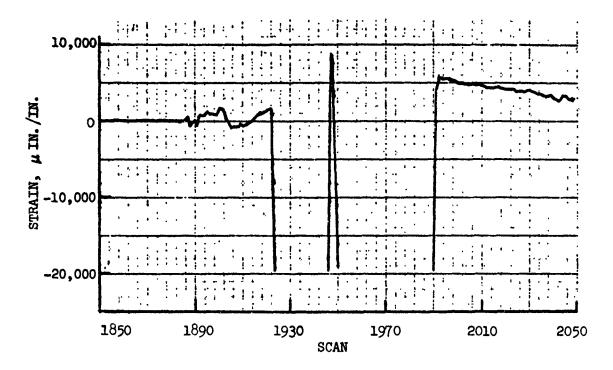


Figure 49. Strain Gage 2 Versus Scan, Test Specimen 6.

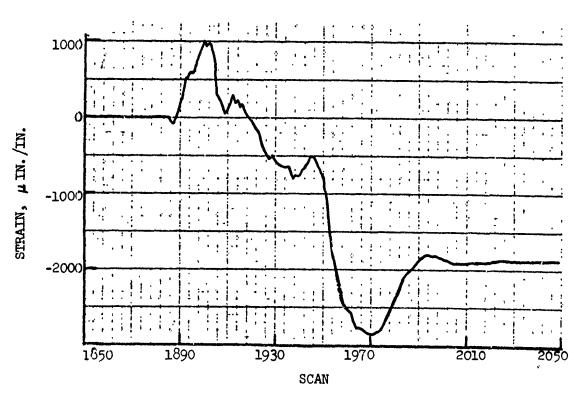


Figure 50. Strain Gage 6 Versus Scan, Test Specimen 6.

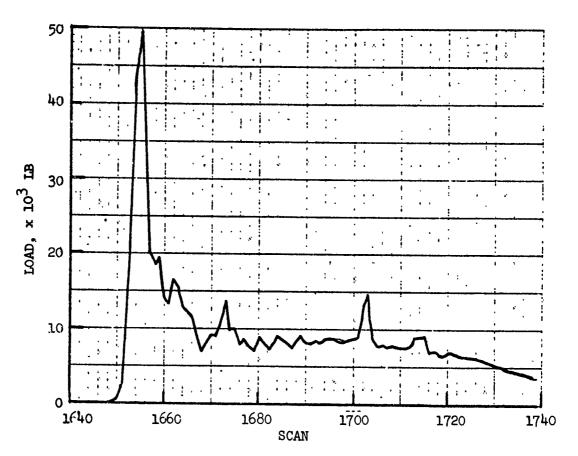


Figure 51. Load Cell (North) Versus Scan, Test Specimen 7.

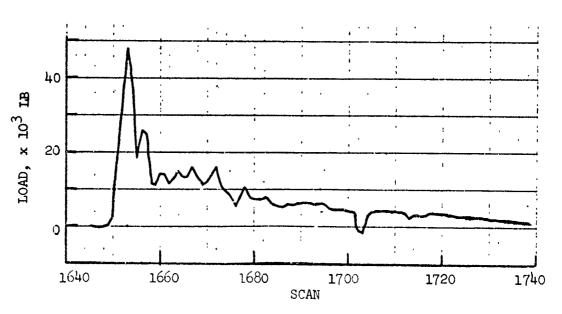


Figure 52. Load Cell (South) Versus Scan, Test Specimen 7.

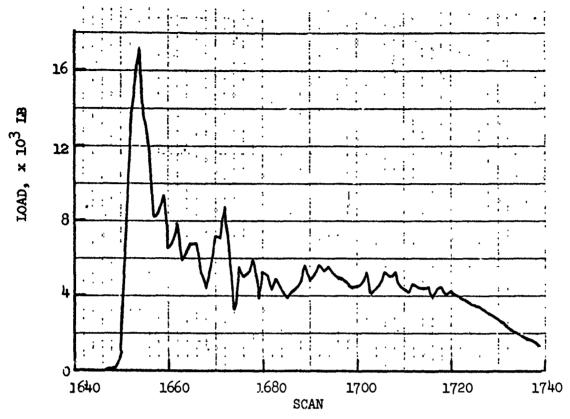


Figure 53. Load Cell (East) Versus Scan, Test Specimen 7.

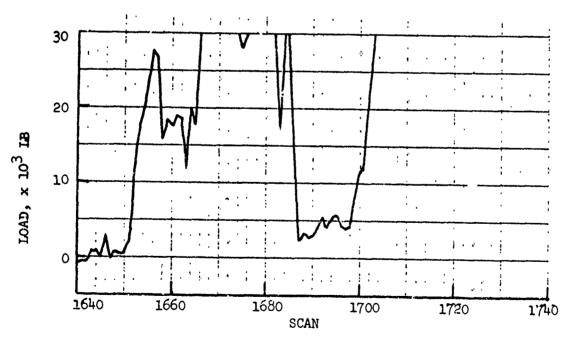


Figure 54. Load Cell (West) Versus Scan, Test Specimen 7.

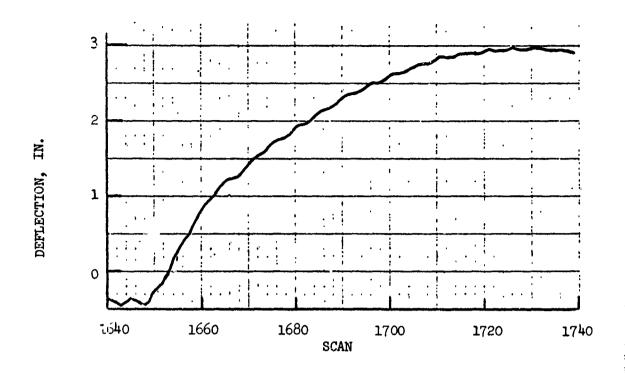


Figure 55. Deflection Versus Scan, Test Specimen 7.

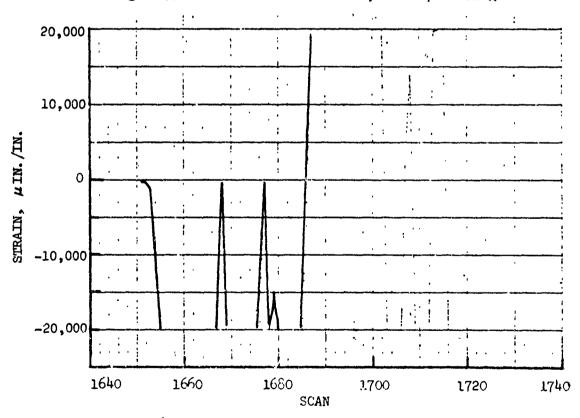


Figure 56. Strain Gage 1A Versus Scan, Test Specimen 7.

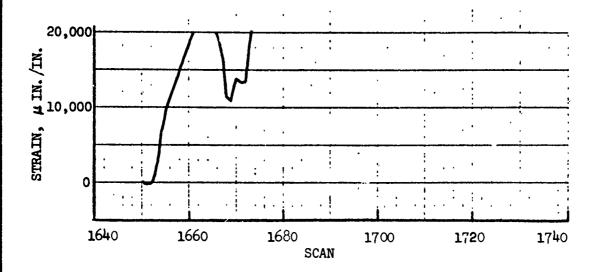


Figure 57. Strain Gage 1B Versus Scan, Test Specimen 7.

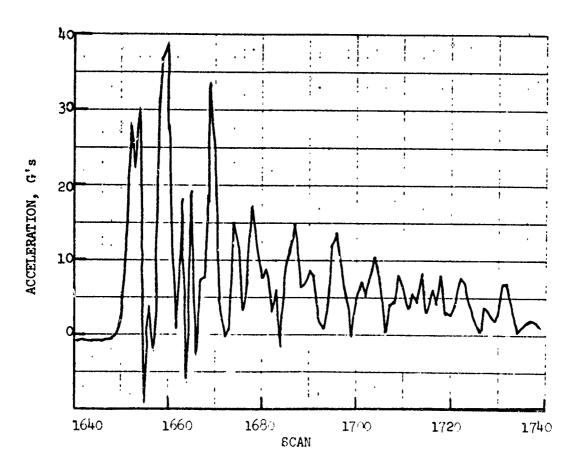


Figure 51. Acceleration (East) Versus Scan, Test Specimen 7.

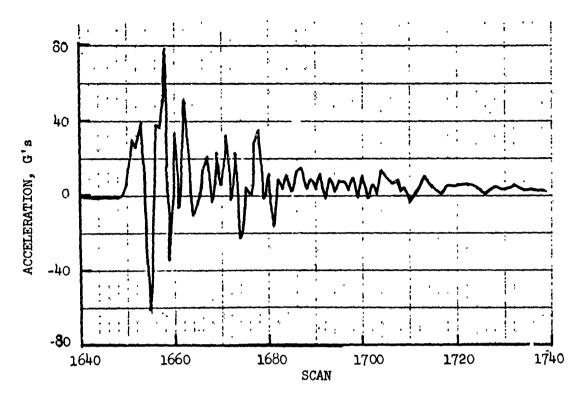


Figure 59. Acceleration (Center) Versus Scan, Test Specimen 7

Figure 60. Acceleration (mert) Versus Scan, Test Specimen 7.

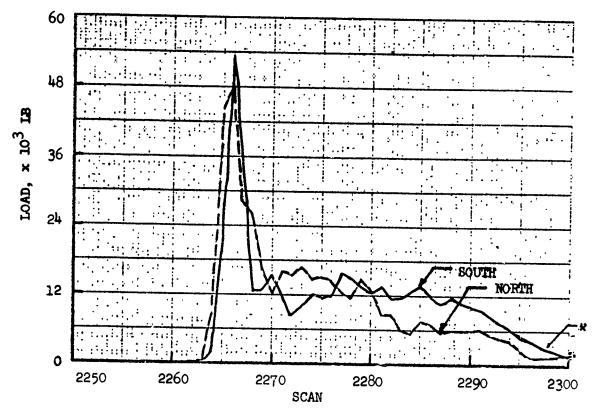


Figure 61. Load Cells (North and South) Versus Scan, Test Specimen 8.

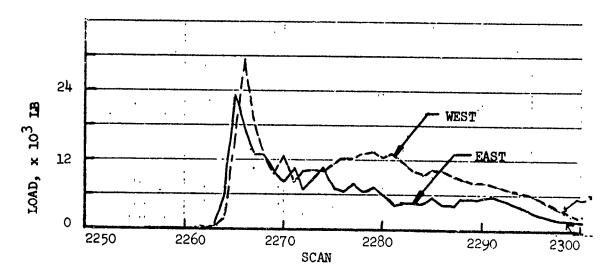


Figure 62. Load Cells (East and West) Versus Scan, Test Specimen 8.

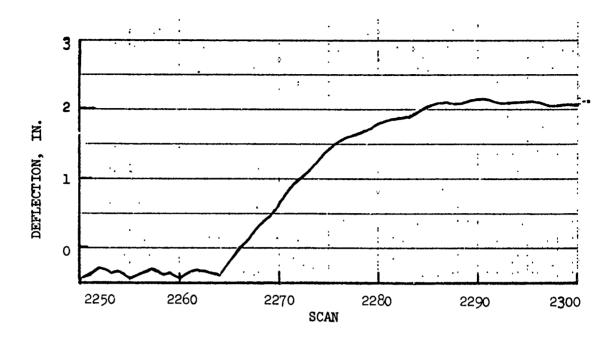
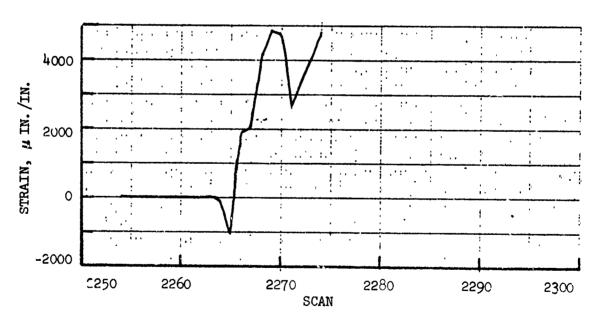


Figure 63. Deflection Versus Scan, Test Specimen 8.



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Figure 64. Strain Gage 1A Versus Scan, Test Specimen 8.

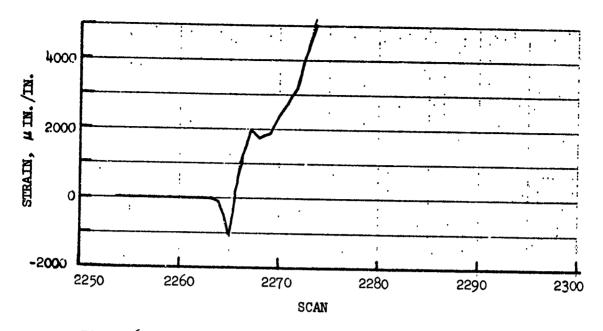


Figure 65. Strain Gage 1B Versus Scan, Test Specimen 8.

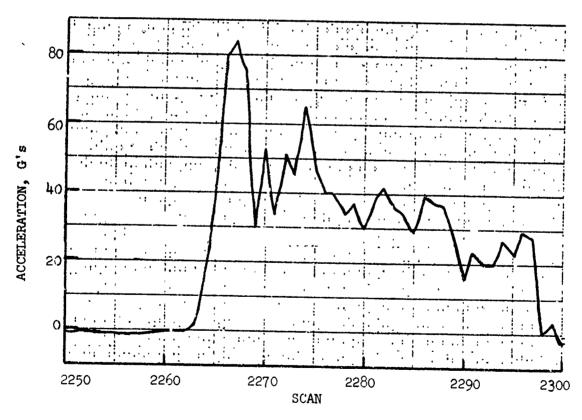


Figure 66. Acceleration (East) Versus Scan, Test Specimen 8.

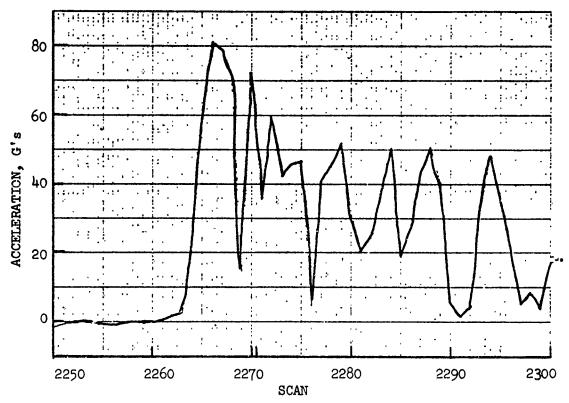


Figure 67. Acceleration (Center) Versus Scan, Test Specimen 8.

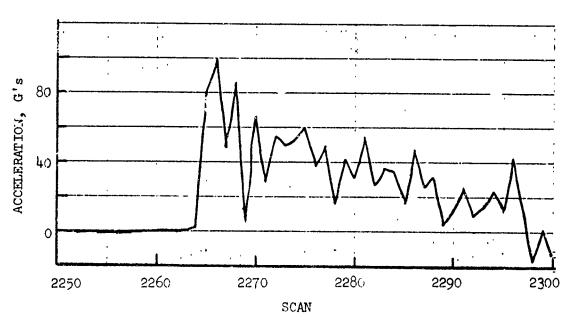


Figure 68. Acceleration (West) Versus Scan, Test Specimen 8.

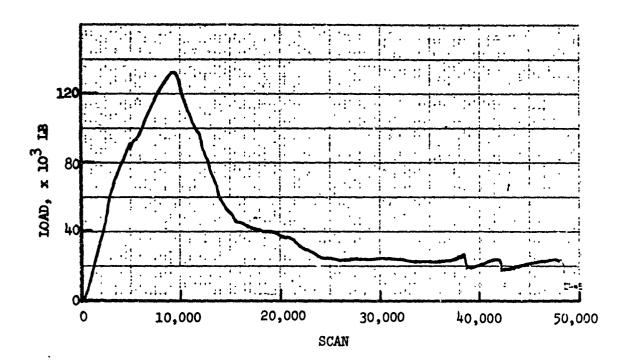


Figure 69. Load Versus Scan, Test Specimen 9.

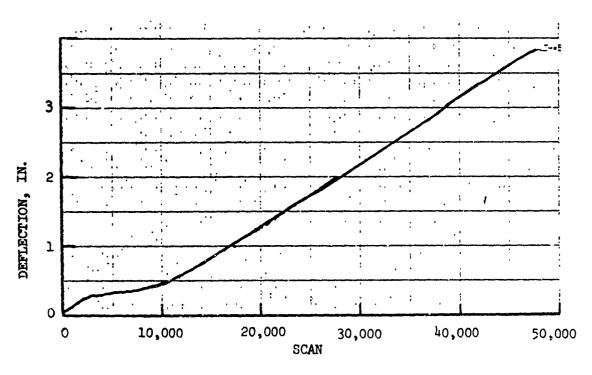


Figure 70. Deflection Versus Scan, Test Specimen 9.

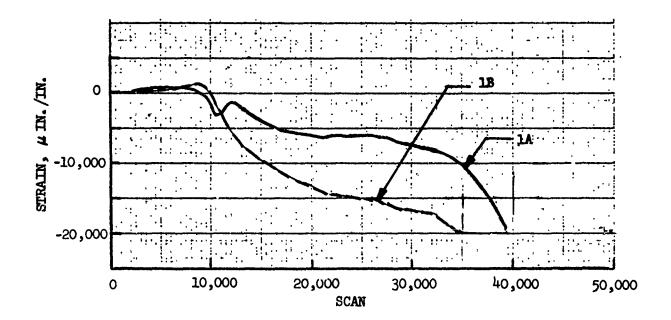


Figure 71. Strain Gages 1A and 1B Versus Scan, Test Specimen 9.

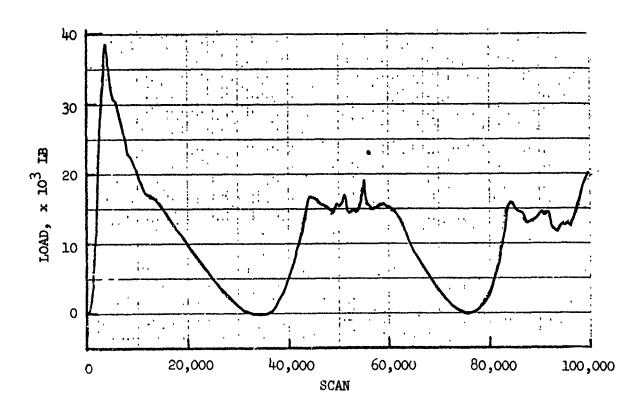


Figure 72. Load Versus Scan, Test Specimen 10.

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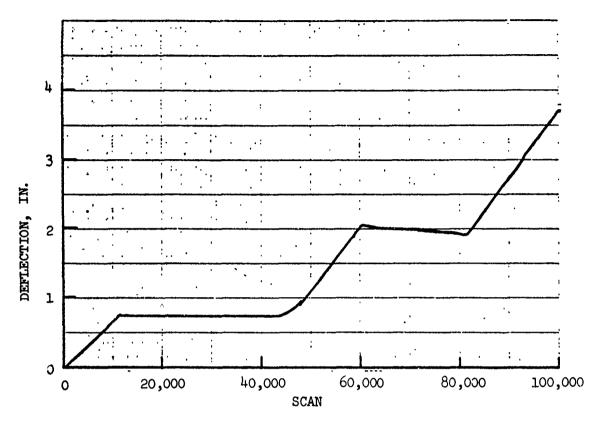


Figure 73. Deflection Versus Scan, Test Specimen 10.

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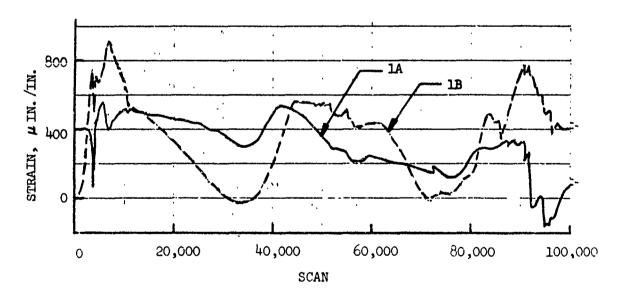


Figure 74. Strain Gages 1A and 1B Versus Scan, Test Specimen 10.

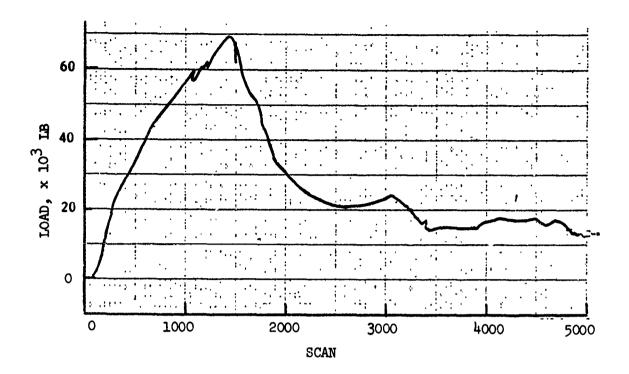
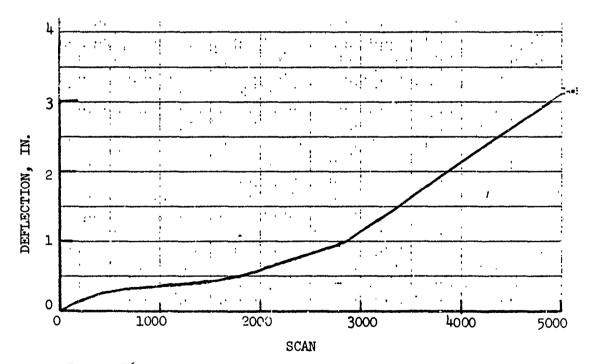


Figure 75. Load Versus Scan, Test Specimen 11.



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Figure 76. Deflection Versus Scan, Test Specimen 11.

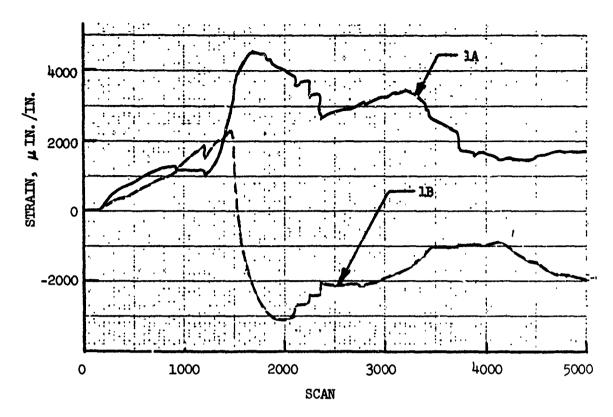


Figure 77. Strain Gages 1A and 1B Versus Scan, Test Specimen 11.

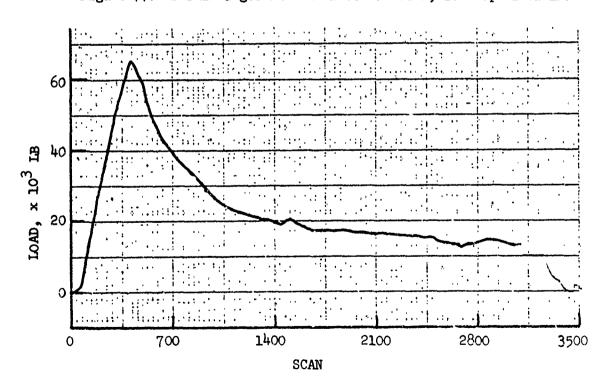


Figure 78. Load Versus Scan, Test Specimen 12.

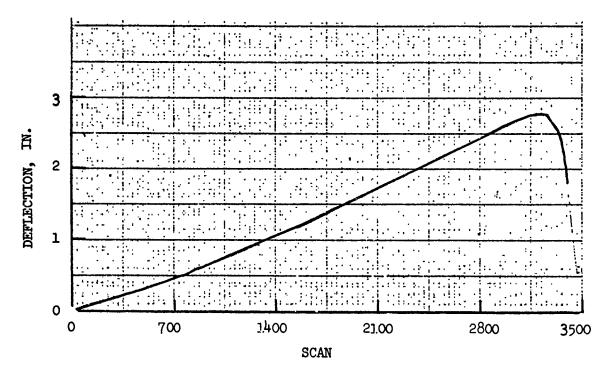


Figure 79. Deflection Versus Scan, Test Specimen 12.

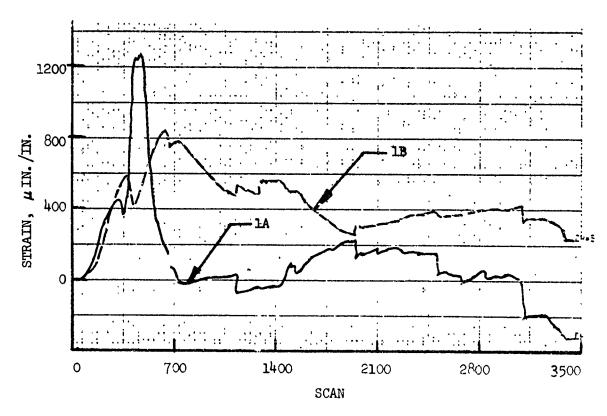


Figure 80. Strain Gages 1A and 1B Versus Scan, Test Specimen 12.

PROGRAM "KRASH" REFINEMENT

GENERAL

Program KRASH was developed as an analytical tool to be used during preliminary phases of design. In Reference 1 the results of a parametric study were integrated into an iterative design procedure by which a tradeoff between potential incremental cost and/or weight versus incremental improvements in crashworthiness capability could be accomplished. The results of that study and a subsequent study for the U.S. Army (Reference 106) showed the potential benefits that could be achieved for designers using a comprehensive but unsophisticated analytical approach. However, as is the situation with rapidly developed advances in the state-of-the-art analytical techniques, there are generally areas which can be simplified for users not thoroughly familiar with the approach.

In an effort to facilitate a designer's usage of program KRASH and to incorporate the results of the literature survey, load sensitivity study, substructure analysis, and tests, the program was revised. In particular, the imput data format was changed for ease of data input and subsequent data changes, as would be required during parameter tradeoff studies. In addition, flexibility was added by the manner in which the Stiffness Reduction factors (KR's) are input. The revised program was run for the following conditions:

- Correlation case 31-52 from Reference 1 (to demonstrate that the revised program format was compatible with previous results: 23-ft/sec vertical velocity combined with 18.5-ft/sec lateral velocity impact).
- Three-dimensional velocity (~40 ft/sec combined velocity impact 27.5 ft/sec longitudinal, 23 ft/sec vertical, 18.5 ft/sec lateral).
- Upper mass penetration into a specified occupiable volume.
- Simplified blade contact.
- Utilization of load-deflection data obtained from the 12 substructure tests, performed during the study, and related to actual fuse-lage structure size.

PROGRAM "KRASH" INPUT FORMAT REVISIONS

The input format changes are divided into three categories:

- Reordered data.
- Standardization of certain inputs.
- Allowance for more general KR curves.

First, the input data is rearranged so that all mass associated data (subscript i) is in one block, followed by all external spring data (subscript ik) and then all internal beam data (subscript ij). Next, standard values are assigned to certain input data items unless otherwise specified. Thus, much repetitive data input is eliminated. The following quantities are given standard values unless otherwise specified:

Quantity	Symbol	Standard Va lue
Angular Momenta	He	0
Euler Angles	φ", θ", ψ",	0
Aerodynamic Lift	lc,	0
Stiffness Reduction Factors	KR _{ij}	1
Failure Deflection	wax ijl	100

Thus, the angular momenta of the masses (He_i) and the aerodynamic lift (lc_i) are normally zero, as are the Euler angles ϕ ", θ ", ψ "_i, relating mass bodyfixed axes to airplane c.g. axes. For linear internal beam elements, KR = 1 for the entire run, so only nonlinear KR data need be input. Similarly, $v_{\text{max's}}$ need only be input for those elements where rupture is expected to occur; for elements not specified, a failure deflection of 100 inches and rotation of 100 radians are assumed. Thus, the nonspecified beams will not rupture during the run.

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The input has also been revised to allow more general KR (stiffness reduction factor) curves. Previously, each KR curve could have only six data points, equally spaced. Now each curve can have up to 15 data points, with any desired spring. This greatly facilitates modeling more complex load-deflection relationships. However, the load-sensitivity study results indicate that for many structural elements a very simple KR representation is adequate. The KR representation for the various load-deflection categories is discussed in Volume I (Design Procedures).

The relationship between the number of masses, internal beam elements and KR tables is governed by the following expression:

Total number of bytes = $235,136 + 1244 \times \text{number of lumped masses (n)} + 1372 \times \text{Internal Beam Elements (IBE)} + 198 \times \text{number of KR tables}$

Lytes =
$$235136 + 260,480 = 495616 = 484K$$

where $1K = 1024$ bytes.

Thus, program KRASH has some flexibility with regard to the size of problems to which it can be applied. Depending on the type of vehicle that is being analyzed the program can be redimensionalized to provide the most practical and economical solution.

The three-dimensional impact mass penetration, simplified rotor blade contact and updated load-deflection data cases are described in Volume I under the section entitled Program KRASH Refinement.

ENERGY BALANCE EQUATIONS

The primary objective of a crash analysis in the preliminary design phase is to determine how to absorb the initial vehicle kinetic energy while maintaining a livable environment for the occupants. This task is greatly facilitated if information regarding the spatial distribution of the energy flow through the vehicle is available. With the objective in mind, energy balance equations are developed. These equations do not alter the existing computational procedures or results; they merely provide additional information to assist in understanding how the initial kinetic energy is absorbed.

The total system energy at any time is given by the following expression:

$$E_{TOT} = KE + PE + SE + DE + CE$$
 (1)

where

 E_{TOT} = Total system energy

KE = Total kinetic energy

PE = Total potential energy

SE = Total strain energy absorbed

DE = Total damping energy dissipated

CE = Total crash spring (external spring) energy absorbed.

The total system energy $E_{\overline{TOT}}$ remains constant during the analysis. The total kinetic and potential energies for each mass over the number of masses (N) are shown in Equation (2).

$$KE = \sum_{i=1}^{N} KE_{i} \qquad PE = \sum_{i=1}^{N} PE_{i}$$
 (2)

The total strain and damping energies are obtained by summing the strain and damping energy for each interval beam element (ij pair) over the M ij pairs.

$$SE = \sum_{i,j=1}^{M} SE_{i,j} \quad DE = \sum_{i,j=1}^{M} DE_{i,j}$$
(3)

 SE_{ij} results from the elastic-plastic behavior of the beam, and DE_{ij} results from its damping properties. The total crash spring energy results from summing the energies for all the individual crash springs (ik pairs) over all the P ik pairs.

$$CE = \sum_{ik=1}^{P} CE_{ik}$$

Referring to Equation (1), at time zero all energies are zero except KE, PE and E_{TOT} . The potential energy is referenced to the ground plane.

After impact with the ground, the kinetic and potential energies decrease; damping, strain and crash spring energies all increase to keep E_{TOT} constant.

In Equation (3), the summations over the ij pairs exclude those ij pairs that are identified in the input as DRI elements, which are described in Reference 1, page 73. This is done because these ij beams and their masses are isolated from the rest of the system; the forces in DRI beam ij drive mass j but not mass i. Also, the summations in Equation (2) for the kinetic and potential energies exclude mass j in a DRI ij pair, since this mass is isolated from the system. Thus, if ij pairs 6-9 and 11-15 are defined as DRI elements, the summations for SE and DE will exclude these ij pairs and the summations for KE and PE will exclude masses 9 and 15.

The kinetic energy for each mass, including translational and rotational components, is simply

$$KE_{i} = \frac{1}{2} \left| vel_{i} \right|^{T} \left| M_{i} \right| \left| vel_{i} \right|$$
 (5)

where

and

Mil is the 6 x 6 inertia matrix for mass i, and velid is a six-element vector of the linear and angular velocity components of mass i in body-fixed axes.

The ith mass potential energy, referenced to the ground plane, is given by

$$PE_{i} = w_{i} z_{i}$$
 (6)

Note that z; is positive downward, measured from the ground plane.

The strain energy in internal beam ij is computed as a continuous summation of the incremental energy contributions from each integration interval.

$$\left(\mathbf{SE}_{ij}\right)_{\text{current}} = \left(\mathbf{SE}_{ij}\right)_{\text{previous}} + \sum_{\ell=1}^{6} \mathbf{F}_{ij\ell} \Delta vb_{ij\ell}$$
 (7)

 F_{ijl} Δvb_{ijl} is the internal beam force (or moment) in the l^{th} direction, multiplied by the incremental beam deflection (or rotation) in the l^{th} direction; this is the incremental strain energy for the integration interval being considered. This straightforward formulation automatically accounts for the complexities of nonlinear, coupled deflections and unloading-reloading behavior, since these are considered in the calculation of $F_{i,j}$.

The damping energy dissipated in internal beam ij is computed in a similar manner:

$$\left(DE_{ij}\right)_{current} = \left(DE_{ij}\right)_{previous} + \sum_{\ell=1}^{6} FD_{ij\ell} \Delta vb_{ij\ell}$$
(8)

FDij! is the internal beam damping force (or moment) in the ! th direction.

The crash spring energy is also computed as a summation of incremental energy changes. The crash spring energy resulting from all the ik springs attached to mass i is

$$(CE_{i})_{current} = (CE_{i})_{previous} - |x_{Ci}|_{Ci} Y_{Ci}..., N_{Ci}| \begin{cases} \Delta x'_{i} \\ \Delta y'_{i} \\ \Delta z'_{i} \end{cases}$$

$$(9)$$

$$(\Delta x'_{i})_{\Delta z'_{i}}$$

$$(4)$$

$$(4)$$

$$(5)$$

$$(5)$$

$$(6)$$

 X_{Ci} , Y_{Ci} , ..., N_{Ci} are the six forces and moments acting on mass i in body-fixed axes, resulting from all ik external springs attached to mass i. These are given by Equations (66) in Reference 1, page 61. The

vector $\left\{\Delta x'_i, \Delta y'_i, \Delta z'_i, \Delta inp_i, \Delta inr_i\right\}$ is made up of the six incremental deflections and rotations of mass m_i, in the same body-fixed axes. The first three terms of this vector are given by a simple rotation transformation of the incremental deflections in ground axes, which are obtained directly from numerical integration of the equations of motion.

$$\begin{cases}
\Delta x'_{i} \\
\Delta y'_{i} \\
\Delta z'_{i}
\end{cases} = |A_{i}|^{T} \begin{cases}
\Delta x_{i} \\
\Delta y_{i} \\
\Delta z_{i}
\end{cases} (10)$$

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The last three terms of the incremental displacement vector in Equation (9) are the incremental changes in the integrals of the angular velocities of mass $\mathbf{m_i}$, $\mathbf{p_i}$, $\mathbf{q_i}$, $\mathbf{r_i}$. These are the incremental rotations of mass $\mathbf{m_i}$ in body-fixed axes for the integration interval being considered.

The negative sign in Equation (9) results from the fact that the forces acting on mass m; rather than the forces within the ik springs are being considered. A positive deflection of spring ik results in a negative force on mass m; The energy calculated in Equation (9) includes the energy dissipated by the sliding of spring ik on the ground with a friction coefficient; hence the forces in that equation include the ground drag loads due to friction.

The crash spring energies in Equation (9) are not yet in the form desired; the crash spring energy due to each spring ik must be separated out of the total crash spring energy associated with mass m_i , CE_i . This is done

simply by substituting Equations (66) of Reference 1 for the $\{X_{Ci}, Y_{Ci}, \dots, N_{Ci}\}$ terms in Equation (9). This reformulates Equation (9) into a function of the individual crash spring forces FSP_{ijk} , where i and k refer to the ik spring and j refers to the direction of the forces on the ik spring. These forces are shown in Figure 10 on page 62 of Reference 1. The final equation for the crash spring energy associated with each spring ik is the following:

$$(CE_{ik})_{current} = (CE_{ik})_{previous} - \sum_{j=1}^{3} FSP_{ijk} \Delta vc_{ij} + TERM_{ik}$$
 (11)

where

$$\left\{\Delta vc_{i}\right\} = \begin{cases} \Delta x_{i} \\ \Delta y_{i} \\ \Delta z_{i} \end{cases}$$

and

TERM_{ik} =
$$FSP_{i31}$$
 ℓ_{i1} $\Delta i \cdot q_i$ - FSP_{i2i} ℓ_{i1} Δinr_i (k = 1)
TERM_{ik} = $-FSP_{i32}$ ℓ_{i2} Δinp_i + FSP_{i12} ℓ_{i2} Δinr_i (k = 2)
TERM_{ik} = FSP_{i23} ℓ_{i3} Δinp_i - FSP_{i13} ℓ_{i3} Δinq_i (k = 3)

This is the crash spring energy to be used in Equation (4) to obtain the total crash spring energy, CE, for the entire vehicle. Note that CE could have been determined more directly by summing the CE; from Equation (9) over all the masses m; However, the CE; associated with each spring ik is calculated so that it can be determined how much each external spring is contributing to CE.

ENERGY BALANCE DATA

The energy balance technique described in the preceding section and the information presented in the following paragraphs describe a type of format which can be of benefit to a designer during the preliminary stage of design.

The output format for the energy balance data is shown in Figures 81, 82 and 83. For a condition of 42-ft/sec vertical velocity impact and using program KRASH with the existing UH-1H helicopter math model consisting of 31 masses and 38 internal beam elements, Figure 81 shows that at time = 0.0 second only contributions to the energy term are kinetic and potential energies. The kinetic energy represent: the 1/2 MV2 terms while the potential energy is a function of the height the respective masses are above the ground at the initiation of impact. The internal beam energy contributions due to strain and damping are zero at this time, as is the external spring crushing energy. Figure 82 shows the energy data output during the run at time = .042 second after impact. At this time kinetic energy has decreased from 85.85% of the total energy to 58.6% of the total energy. Similarly, the potential energy has decreased from 14.15% to 8.96% of the total energy. At this time crushing energy, strain energy and damping energy as a percent of the total energy have increased to 9.8%, 7.5% and 15.2%, respectively. Furthermore, Figure 82 shows the following:

- distribution of kinetic energy and potential energy by mass item
- distribution of strain energy and damping energy by beam element
- distribution of crushing energy by external spring element

The negative terms in the damping energy output for beam elements 2 and 4 are of the order of less than .0003 percent of the total energy. The values are obviously insignificant and are considered to reflect damping inherent in the integration routine.

Figure 83 shows a summary of the energy data which is presented at the end of the computer run. The summary shows for each time increment the total energy and the amount and percentage contribution from kinetic, potential, strain damping and crushing. The data in the summary shown in Figure 83 indicates that for the illustrated case the vehicle rebounds at approximately .064 second after impact. The kinetic energy reduces from impact until .064 second after impact, then starts to increase again. The summary data presented in Figure 83 also shows that crushing energy reaches a maximum of 51.73% of the total energy at .062 second after impact and that strain energy reaches a maximum of 21.93% of the total energy at .076 second after impact.

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Figure 81. Energy Balance Data Output at t = 0.0 Second

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	PERCENT		0.0	0.79	1.14	1.19	6.63	31.5	51.65	51.73	54.05	10.57	26.61	18.23	
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	PERCENT		0.0	3.16	8.57	13.99	15.17	15.40	16.90	17.25	17.61	18.50	39.06	19.55	
DAMPING	ENERGY		0.0	1.03025	2.603E5	4.602E5	5.011E5	5.153E5	5.584E5	5.698E5	5.815E5	6.112E5	6.298E5	6.467E5	
	PERCENT		0.0	1.5	3.6	6.74	7.38	9.33	16.8	17.08	18.54	21.03	21.93	19.53	
STRAIN	ENERGY		0.0	4.883E4	1.145E5	2.221E5	2.432E5	3.081E5	5.231E5	5.698E5	5.815E5	6.112E5	6.298E5	6.467E5	
	PERCENT		14.15	12.85	11.57	10.30	9.17	8.2	1.67	1.64	7.63	7.75	8.04	8.31	
POTENTIAL	ENERGY		4.608E5	4.186E5	2.78LE5	3.395E5	3.028E5	2.708E5	2.53335	2.522E5	2.520E5	2.560E5	2.659E5	2.748E5	
	PERCENT		85.85	81.T	75.21	67.8	61.66	35.38	4.5	6.30	6.11	12.15	24.36	34.38	
KINETIC	ENERGY		2.796E6	2,662B6	2.458E6	2.234E6	2.038E6	1.168E6	2.62EE5	2.C80E5	2.017E5	1016E5	8.053E5	1.13定5	
	PERCENT		180.	100.04	100.35	101.15	101.39	101.41	101.4	101.4	101.47	101.46	101.46	101.53	
TOTAL.	ENERGY	*	3.25606	3.259D6	3.26906	3.29616	3.30306	3.303D6	3.30306	3.30306	3.30306	3.30306	3.305D6	3.30706	
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Computer Printout is Every .002 Second. For Illustrative Purpose an Abbreviated Computer Printout is Shown

*7 denotes double precision

Figure 83. Energy Data Output Summary.

REVISED USER'S GUIDE

This section describes all the input necessary to run the program, and the output available from the program.

Input

The input data format is described in detail in this section and is shown in Figure 84. Unless otherwise specified, all quantities are input in inch, pound, second, and radian units. For cards 0102 and following the input format is 6E12.0 unless otherwise noted. Each card has 6 fields; each field is 12 columns wide. As an example of this format, the number 126.08 can be input in the following ways:

		ı	2	6	•	0	8		
Γ		1	2	6			8		

	1		2	6	0	8			E	2
Γ	1	2	6	0	8	•		E	1	2

Blank columns are treated as zeros. When the E format is used, the exponent must be right justified in the field. Integer formats with field widths of 1, 2 or 3 are also used. The numbers in these locations must be right justified integers. Sequence numbers in columns 77 through 80 should be used corresponding to those shown in the input format to facilitate deck assembly and changes. All tabular input is linearly interpolated between input values and extrapolated beyond the two end values, if necessary.

Card OlOO - This card contains the title for the case being analyzed. All text material on card OlOO is reproduced at the top of every page of the output and on every plot.

<u>Card OlOl</u> - N is the total number of lumped masses. The maximum allowable number of masses is 80. Δ Print/ Δ t is the integer multiple of Δ t at which output is printed. Δ t is the numerical integration time interval. ε_{max} is the time span being analyzed.

Cards 0102 through 0104 - These cards contain the overall vehicle initial conditions. \dot{x}_G , \dot{y}_G , and \dot{z}_G are the ground axes components of the initial c.g. velocity. p', q', and r' are the c.g. coordinate system components of the initial angular velocity of the vehicle. p' is the roll velocity, q' the pitch velocity, and r' the yaw velocity. ϕ' , θ' , and ψ' are the Euler angles relating the initial position of the vehicle to ground coordinates. ϕ' is the roll angle, θ' the pitch angle, and ψ' the yaw angle. z_G is the negative of the initial vehicle c.g. height above ground. If this input is zero (blank), the initial condition subroutine computes a value of z_G so that the lowest extremity of the vehicle is .1 inch above the ground.

Ca ds 0201 through 02XX - N of these cards are used to input the weights (nct masses) of the N lumped masses.

Cards 0301 through 03XX - N of these cards are required. Each card inputs the six moments and products of inertia for the ith maxx, i = 1, \hat{z} , ..., N.

<u>Cards C+Ol through 04XX</u> - N of these cards are used to imput the coordinates of the N lumped masses. x_i^n is the Fuselage Station (increasing aft), y_i^n is the Butt Line (positive left), and z_i^n is the Water Line (increasing upward).

<u>Card 0500</u> - NI is the number of masses having nonzero He_{xi} , He_{yi} , He_{zi} (angular momenta) or ϕ_{i}^{w} , θ_{i}^{w} , ψ_{i}^{w} , which are the Euler angles relating the c.g. axes to the ith mass body fixed axes. The above quantities are normally zero, so only nonzero values are input on cards 050I through 05NI. i₁, i₂, ..., i_{NI} are the actual i's or mass numbers that have nonzero input data for any of the above quantities. NI and i through i_{NI} are input in integer format I2. If NI equals zero, cards 0501 through 05NI are not input; however, card 0500 is always required. If NI equals zero, a bland card for card 0500 is input.

Cards 0501 through 05NI - As noted above, these cards are used to input any nonzero values of He_{xi} , He_{yi} or ϕ_i'' , θ_i'' , ψ_i'' . One card is used for each mass i having nonzero input. The masses are ordered according to the sequence specified on card 0500. He_{xi} , He_{yi} , and He_{zi} are the body axes components of the angular momenta of masses m_i , die to rotation of internal masses within m_i . These are normally zero. ϕ_i'' , θ_i'' , " are the Euler angles from the c.g. axes to the ith mass axes. If the ith mass body-fixed axes are parallel to the vehicle c.g. coordinate axes. which is usually the case, these are all zero. Note that if any nonzero values are input, then θ_{ij} and ψ_{ij} on cards 0901 through 090M must be input.

<u>Cards 0600 through 06NI</u> - NI is the number of masses having nonzero l_{c_i} , which are the aerodynamic lift constants used in Equation (5) on page 30 of Reference 1. i_1 . i_2 , ..., i_{NI} are the actual mass numbers having nonzero l_{c_i} : If all l_{c_i} are zero, card 0600 is input as a blank card and cards 0601 through 06NI are not input. One card is used for each nonzero l_{c_i} input, with the ordering defined on card 0600.

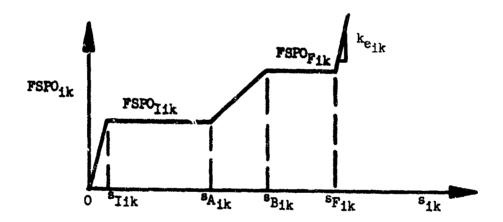
Cards 0701 through 070p - As many cards as necessary are used to input data for the external springs. The maximum allowable number of external springs is 50. Each spring is identified by an ik pair, where i defines the mass to which the spring is attached and k defines the direction of the spring. k = 1, 2 and 3 correspond to springs in the x, y and z directions, in mass i body-fixed axes. \bar{l}_{ik} is the free length of the external spring ik. l_{ik} is positive if it radiates out from mass m_i in

the positive direction of the ith mass body axes; l_{ik} is negative if it radiates in the opposite direction. Springs in both directions are not allowed. mu_{ik} is the friction coefficient between the ground and the end of the ik spring. ke_{ik} is the linear unloading stiffness and also the bottoming stiffness for the ik spring.

If no external springs are used, card 0701 is imput blank and the next card input is 0901.

Card O7XX - A blank card in this location is required to terminate the above ik external spring list.

Cards 0801 through 080p - External spring load-deflection curve parameters are input on these cards, one card per ik spring, ordered as in the ik list on cards 0701 through 070p. The program is written so that a table of the following form is input:



This table is defined by six parameters S_{Tik} , S_{Aik} , S_{Bik} , S_{Fik} , FSPO Iik, and FSPO which are input on cards 0801 through 080p.

Cards 0901 through 090M - These cards contain the Euler angles ϕ_{ij} , θ_{ij} , and ψ_{ij} for all internal beams ij. The beam interconnections are defined by the i's and j's irput. i must be less than j, but there is no other requirement on the ordering of the ij pairs. ϕ_{ij} is always input; θ_{ij} and ψ_{ij} need not be imput if ϕ_{i} , θ_{i} and ψ_{i} on cards 0501 through 05NI are all zero. In the latter case, θ_{ij} and ψ_{ij} are computed in initial conditions. ϕ_{ij} will normally be zero. The maximum allowable number of internal beams is 100.

Card O9XX - A blank card is required here to terminate the ij pair internal beam list.

Cards 1001 through 1006 - These six cards are used to imput the 6 x 6 linear stiffness matrix $|K_{ij}|$ for the first ij beam, listed on card 0901: Each card imputs one row of the matrix.

Cards 1007 through 1XXX - As many cards as necessary are used to input all the remaining 6×6 $|K_{ij}|$ matrices. These matrices must be ordered the same as the ij pairs are ordered on cards 0901 through 090M.

<u>Cards 2001 through 200M</u> - These cards are used to imput the \overline{C}_{ij} for all the ij pairs defined on cards 0901 through 090M. \overline{C}_{ij} is the damping ratio (damping/critical damping) for the isolated system consisting of masses m_i and m_j connected by beam ij. Values of \overline{C}_{ij} between .01 and .05 are generally representative of the structural damping.

Cards 2101 through 210q - These cards are used to specify which beam elements (ij pairs) and directions (1) have nonlinear stiffness properties. I varies from 1 to 6 corresponding to the x, y, z, ϕ , θ , ψ , directions in beam axes. Only beam elements and directions having nonlinear properties (KR \neq 1) are input. For each ijl combination listed, a table of KR (stiffness reduction factor) versus deflection is input on the following cards. The NP's are the number of points in the following tables. For all itjl combinations not listed, KR = 1 for the entire run. The maximum allowable number of ijl combinations (nonlinear KR tables) is 80. The maximum allowable number of points per table is 15. For a completely linear analysis, card 2101 is blank and the next card input is 3001.

<u>Card 21XX</u> - A blank card is required here to terminate the ijl list of nonlinear beam/direction combinations.

Cards 2201 through 22NP₁ - These NP₁ cards are used to input the deflection (vb_{ijl}) versus KR_{ijl} table for the ijl combination listed on card 2101. KR_{ijl} normally starts at 1.0 zero deflection (or rotation), corresponding to linear behavior, and varies in any desired manner thereafter to define a general load-deflection curve. Note that KR is not the actual load, but rather the derivative of load with respect to deflection, i.e., the slope of the load-deflection curve.

Cards 22X1 through 2XXX - As many cards as necessary are used to input the KR_{ijl} vs. vb_{ijl} tables for the remaining ijl combinations listed on cards 2100 through 210q above. Note that each table can have a different number of data points NP_i .

Cards 3001 through 300r - These cards are used to identify which internal beam elements it have nonstandard maximum deflections for rupture (v_{maxijl}). The standard deflections built into the program are 100 inches for deflections and 100 radians for rotations. These numbers were deliberately chosen to be very high so that rupture would not occur unless reasonable v_{max's} were specified. If no nonstandard v_{max's} are to be input, card 3001 is input blank, and the next card input is 5001.

<u>Card 30XX</u> - This blank card is required to terminate the preceding ij list for nonstandard v_{maxi,jl}.

Cards 3101 through 310r - These cards are used to input the nonstandard $v_{maxijl's}$, one card for each ij beam specified on cards 3001 through 300r. Each card inputs the six $v_{max's}$ for one ij beam, ordered x, y, z, ϕ , θ , ψ in beam axes. v_{maxijl} defines the maximum allowable deflection before rupture occurs. After rupture, all forces in beam ij go to zero, regardless of the direction in which the rupture occurred.

Cards 5001 through 5025 - These cards are used to specify the time history output plots desired. The only input for these cards is either a 1 or a blank. A 1 results in the output of a time history plot for the response quantity indicated; a blank results in no plot for that item. For example, a 1 in the 13th column of card 5003 specifies that a time history plot of z₁₃ is to be generated. For cards 5001 through 5012, the column number in which the 1 is input indicates which mass i is desired. For cards 5013 through 5024, the column number in which the 1 is input indicates which ij pair is desired, where the ij pairs are ordered as on cards 0200 through 02XX.

Plots are available for the displacements, velocities and accelerations of all the lumped masses, all the external spring compressions S., and all the beam ij total deflections/rotations (vb.,) and forces/moments $(F_{i,j})$. The latter two items are in beam ij $axe^{\frac{1}{5}J}$. Also available are plots of the DRI (Dynamic Response Index). These are identified by the DRI ij element, as described on card 7000 below. The plot variables are labelled automatically, and the plot scales are chosen automatically. The user merely has to specify which plots are desired. Up to 150 plots can be requested per run. Thirty thousand data points are stored for plotting, with the plot time interval depending on run time and number of plots requested. Thus, if the maximum of 150 plots are requested, 200 points will be saved to generate each plot. If the maximum run time is .2 second, this will give one data point every .001 second. For the types of problems analyzed, this appears to be a marginally acceptable resolution. Requesting fewer plots automatically increases the number of points per plot and, hence, the resolution.

<u>Card 6000</u> - P is the mass to be used to locate the mass penetration control volume. The format is I2. If no mass penetration calculations are required, this card is input blank and card 6001 is not required.

<u>Card 6001</u> - xn, xp, yn, yp, zn, and zp are the six distances (all positive), measured from the control mass P to the six sides of the control volume. These are measured along the positive (p) and negative (n) body-fixed axes for mass mp.

Card 7000 - This card is used to specify which internal beam ij elements, if any, are to be treated as DRI (Dynamic Response Index) elements. For example, if the 15th ij pair listed on cards 0901 through 090M is to be a DRI element, then a 1 is imput in column 15 of card 7000. For DRI beam elements, the forces in beam ij drive mass j but not mass i. To understand the physical significance of this, refer to Reference 1, page 73, which explains the concept of a DRI.

Cards 8000 and 8001 - Cards 8000 and 8001 are used to specify il (mass-direction) pairs for which it is desired to input a time history table of acceleration. Thus, any mass can be driven by a specified acceleration in any direction. The 6 directions are ordered x, y, z, ϕ , θ , ψ in mass i body-fixed axes. The format is 24 (I2I1), where the 1 is right justified in the field of 3 and the i is right justified in the left two columns of the field of 3. A maximum of 48 input tables are allowed. For example, driving mass 3 in the y direction would be specified as |032|.

When any acceleration table is imput, that acceleration replaces the normally computed acceleration, so it becomes a driving force to the system. If less than 24 il pairs are specified, only card 8000 is required (omit 8001). If 24 or more il pairs are input, cards 8000 and 8001 are both required (if exactly 24 il pairs are input, card 8001 will be blank, but must be input).

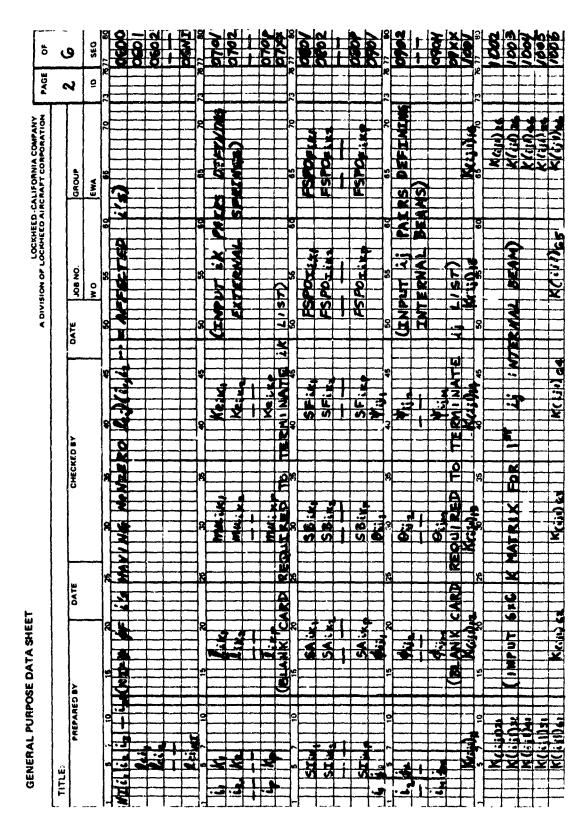
<u>Card 8010</u> - N_1 is the number of data points in the following acceleration time history table, applicable to the first il pair on card 8000. The maximum allowable value of N_1 is 50.

Cards 8011 through $801N_1 - N_1$ cards are used to input the time history table of acceleration for the first il pair. Each card inputs one time and the corresponding acceleration.

Cards 8XXO through 8XXN_q - The remaining acceleration time history tables are imput on these cards, in the format described above. The tables are ordered according to the sequence of il pairs defined on cards 8000 and 8001.

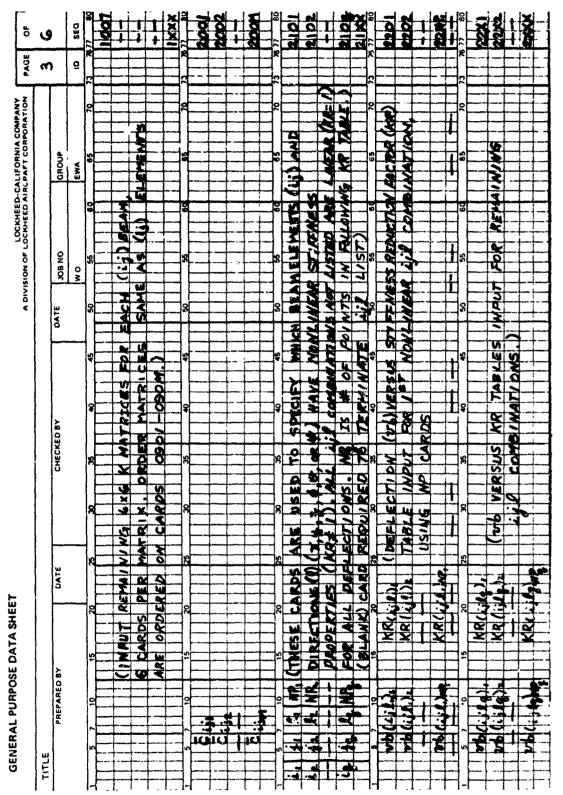
Figure Co. Input Data Format.

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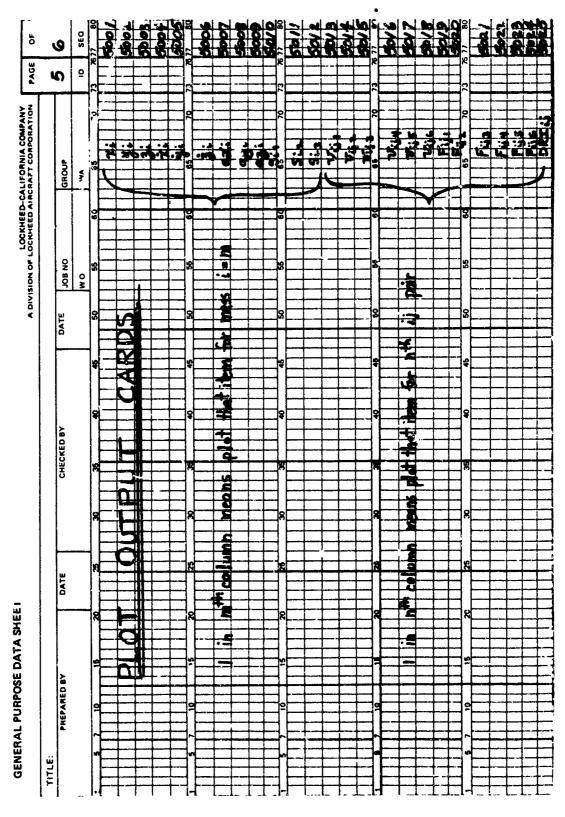
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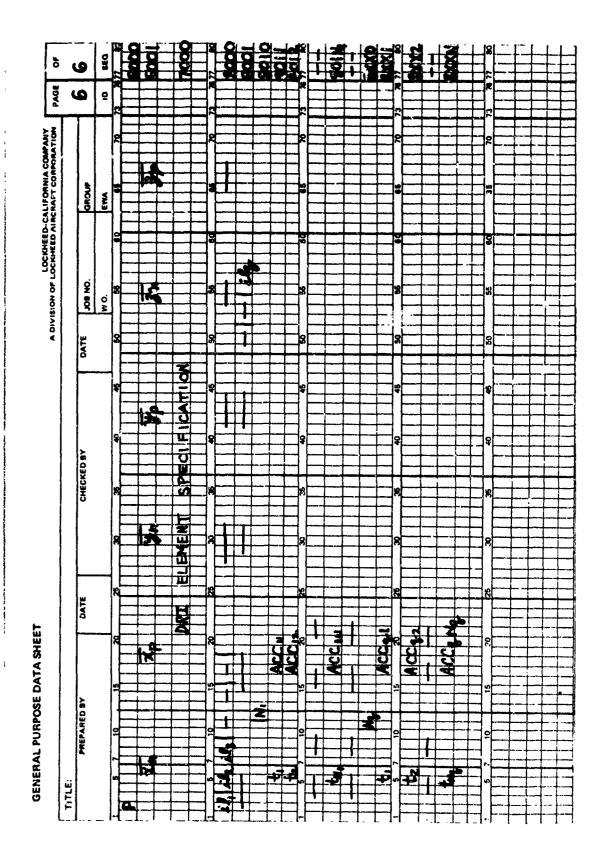
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Print Output

First, all the input data is printed out, with self-explanatory identifying titles. Next, at each print time (= $\Delta Print/\Delta t \times \Delta t$), the data shown in Figure 85 is printed out. At the top of each page, the case identification data is printed out (from input card 0100). Then the current value of time t is printed.

Next, for each of the N masses, five lines of data are output. x_i , y_i , and z_i are the ground coordinates of m_i . \dot{x}_i , \dot{y}_i , and \dot{z}_i are the ground axes components of the velocity of m_i . \dot{u}_i , \dot{v}_i , \dot{w}_i are the time derivatives of u_i , v_i , and w_i . (These are not equal to the acceleration of m_i , as can be seen from Equation (81), Reference 1.) XACCEL, YACCEL are the body axes components of the acceleration of m_i .

 ϕ , θ , and ψ are the Euler angles defining the attitude of body m_i , in roll, pitch and yaw. ϕ_i , θ_i , and ψ_i are the time derivatives of ϕ_i , θ_i , and ψ_i . \dot{p}_i , \dot{q}_i , and \dot{r}_i are the body axes components of the angular velocity of m_i ; they are the velocities in roll, pitch, and yaw, respectively. p_i , q_i , and r_i are the body axes components of the angular acceleration of m_i .

Following that output, the running time sums of the internal forces $\{F_{ij}\}$ (Equation (27), Reference 1) are printed out; the six forces and moments for each ij pair are printed on a line preceded by the identifying i and j. Next, the running time sums of the beam deflections $\{v_{bij}\}$ (Equation (26), Reference 1) are printed out in the same format.

Finally, the external spring compressions s_{ik} (Equation (47), Reference 1) are printed out. Each line starts with the i, followed by the s_{ik} for k=1, 2, 3. Only values of i for which a spring is input are shown.

During the course of the run, if any ruptures or control volume mass penetrations occur, the appropriate information is printed out. When a rupture occurs, the word RUPTURE is printed, followed by four items:

- 1. The ij pair that ruptured, where the numbering corresponds to the ordering of the ij pairs as input on cards 0200 through 02XX.
- 2. The 1 (from 1 to 6) indicating in which direction (in beam axes) the rupture occurred. These are ordered x, y, z, ϕ , θ , and ψ . See Figure 24 (Reference 1) for the directions of the beam axes.
- 3. The vb_{ijl} at the time of rupture. This is the total deflection, in beam axes, in the direction of the rupture.
- 4. The v_{max}; which defines the maximum allowable beam deflection in the lth direction. This is an input constant.

If a control volume mass penetration occurs, the mass which penetrates and the time are printed out. At the end of the run, the ruptures and mass penetrations that occurred during the run are summarized in tables for ready reference.

Sample Cases

Two sample cases are presented. The first sample case represents a combined vertical and lateral impact condition. The vertical impact velocity is 23 ft/sec and the lateral impact velocity is 18.6 ft/sec. The mathematical model is the 31 mass representation shown in Volume I, Figure 6 and Table VI. The sample output for one time period is shown in Figure 85. The input data, which follows the User's Guide Input format (Figure 84), is presented in Figure 86.

Sample output plots are provided for the engine mass vertical velocity, engine mass vertical acceleration and engine mount support vertical deflection in Figures 87, 88 and 89, respectively. Figures 90, 91 and 92 provide similar sample plots for the lateral velocity, lateral acceleration and mount lateral displacement. respectively. Figures 93 and 94 show the forward and aft DRI plots, respectively, for the comoined vertical and lateral impact velocity case. The engine vertical responses shown in Figures 87 (velocity), 88 (acceleration) and 89 (displacement) can be compared to Figures 17, 18 and 19 in Volume II, Reference 1 to show that the revised program KRASH performs in the same manner as the final submitted program KRASH from the previous study (Reference 1).

The second sample case provided in this section is the three-dimensional velocity impact condition. The initial c.g. velocity for this case is a combined 40 ft/sec velocity in which the velocity vector components are:

$$V_x = 27 \text{ ft/sec}$$
 $V_y = 18.6 \text{ ft/sec}$ $V_z = 23 \text{ ft/sec}$

The initial vehicle attitude is flat (zero roll, pitch and yaw angles), and the initial angular velocities are zero. Figure 101 (page 140) in Volume I shows the mathematical model representation for this condition. The model consists of 32 lumped masses. It is the same model used in the previous sample problem except for the extra mass location which represents the rotor blade during a potential mass penetration (of the upper cabin) condition. The input data for this sample problem is provided in Figure 95. A sample output for one time period is shown in Figure 96. Figures 97 through 99 show the engine mass vertical, lateral and longitudinal velocities, respectively. Figures 100 through 102 present the engine mass vertical, lateral and longitudinal accelerations, respectively. Figures 103 through 105 show the engine support mount vertical, lateral and longitudinal deflections, respectively. The forward and aft DRI's are presented in Figures 106 and 107.

Program Listing

A complete listing of the computer program KRASH, including subroutines, is presented in Figure 108.

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		80 - 10 1C - C B	-9.84533D 01		1.000620-01	4.564350-03	2-45445D-L1
		3.4740	-1.315060 02		-1. HOBBOD D1	-4.855566-01	1.472730 01
		1 - Afagoz • 1 -	-9.319300-06-	7.288210_00			:
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					0-222340-01	3.239300-01	1.580260-01
		10 34 447 1	10 028461.3	-1.28 (40 02	6-18146-01		1.649530-01
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MASC	•		3.075200 01		20-00F: *-	1.066320-02	6.31572D-02
		882400			5 3010-01	6.652860-01	
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RUM 31-52 DROP TEST CORRELATION 206/186 ENGINE

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		-7.03670D OL		1.375520 00	5.797620-01	4-234240-01
			-1.421980 02	1.371250 00	5.613360-01	4.400510-01
	-7.337330 02 -1.998530 03	-4.533550 02 -6.940330-01	2.907300 00	2.233270 00	5.829820 01	-6.552810-01
MASS 10	-2-340246 01	-2.471910 01	10 013404-1-	-1-872 480-02	1.204480-02	0-048084-6
	7-161460		048850	A . 710 24 7 - 01	2.612340 00	10-004003-1
			05 70 80	10-786929-5		0-090879-6
	4. 3088VD 07		.324950	-3.101970 01	1.414450 00	7.304090 00
	5.786340-01		-3-641340 00		- 1	
MASS 11	-1.608210 00	-2.910540 01	-1.438820 01	-2.069040-02	1.628410-02	2.342370-0
	6.995t-D 00	-1.126740 02		7.104940-01	3.743203-01	9-124290-0
		000640*	-1.016400 02	7.090csD-31	3.023670-01	9.750740-02
	3.443150 02	-1.223560 02	1.040640 03	1.837500 01	6.261370 00	1.954090 01
	10-012715-01	-5.514580-02	2.488920 00			
MASS 12	6.487680 01	1.739160 01	-1.750720 31	-7.567160-02	5.052170-02	1-602980-02
	-3.061290 00	-1.150850 02	-2.251460 02	-3.454520 00	4.635010-01	1-345040-01
		-9.762490 01	-2.33179D 02	-3.966370 00	4.519850-01	1.690790-01
	-1.471570 03	5.668720-01	2.265710 00	2.466370 02	-3.456910 01	
M455 11		-7.246030 01	10 057805-1-	- 456 33D-03	4 21047000	0.000
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	-6.120580 02	3.344520 00	-5.114460 03	-7.6r339D 01	-5.613970 00	-4-43988C 0
MASS 14	-2.474030 01	3.352015 01	-5.710330 00	-2-394670-02	-2.063170-02	4-393490-03
		-5.424960 01		9.730470 00	-3.449400-02	-3.882300-01
	-3.114330 00				-2.319020-02	-3.006605-01
	2.766320 01	-2.393920-32	20 002065.6	3.213260-03	1.239040 00	3-7007-6
MASS 15	•	-1-129530 92	-1.527550 01	-3.8 76 36B-01	-3-485620-02	-2.027070-0
	1.246400			-7.200280 00	-1 - 355450-01	214320-02
			-3-114660 02	-7.201830 00	-1.095410-01	-0.025910-02
	1.578-70 01		-2.492380 03		-2.130350-01	-1-480300-16
	34664760-02	-3.780250-01	9.250620-01			
MASS 16	6.774830				4. 9401 70-02	1.92709D-02
			-3.608440 nl	-3.043130 00	1.761520-01	1.180800
		-1.052020 02	-3.902260 01	-3.0800TD 00	1.279720-01	-5-784450-0

Figure 85. (Continued)

PSI PSIDOT RDD		1.501400-02 -1.656590-01 3.747480-01	-7.829350-03	-5.781320-02	2.368467-19	1.961530-02	-2.42729-02	1.397460 01	1.436410-02	3.309120-01	3-181629-01	- 1	1-648300-02	10-022202	2.8226.20 01		2.406060-02	10-00-00-0	4.297916 01	1.809590-02	1-796260-01	- 101970 00		2.372530-02	70-04-04-1-	00 QL070%*C
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141 1007		-4-29066D-01 -4-94834D 00 -4-53965D 00		-5.31674D 00		-3.997890-02	- 000 920 00 - 000 920 00	1.205788 02	-1 -64364D-02	6-511850-01	-1.407040 00		-1-688470-02	3.157020-01	-3.584840 01		-2-14918D-02		-1.277250 01	-1.774360-02	5-712470-01	-2.20849D 01		-5.191360-02	2.025430 00	2.020119 Oct
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MASS 26	10 044078.45	-7.99067D 01	1 -2.385880		-7.13407D-02	8-8-10-03-03	2-200410-02
			1	20	5.221490-01		1-042850-01
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14 5 2 2 2 2 4 4 4 4 4 4 4 4 4 4 4 4 4 4	6- 7240AB 01	-2.788650 01	1-2-548030	5	-4.227090-02	4.455880-02	2.07232102
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MASS 28	-2.37388D D1				-2.237010-02	6.56122D-03	2.190770-02
	-4.41362D 01		-		3.467860-01	3.344450 00	2.548860-02
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			1	70	3.535020-01		1.329320-02
		.070510		7	3.534090-01	3.376870 00	8.455300-02
	1.505480 02	-8.01865D 02 -1.93996D 00	2 -8.253460 0 -2.112310	03	8.524400 01	6.534030 01	4.482140 00
MASS 31	6.677720 01	-2.615140 01	1 -3.370865		-3.259630-02	5.872520-02	2.212480-02
	-1.452480 01	-1.5900ED 02	•		-2.29370D 00	1.695710 00	-4.603280-02
			-	20 0	-2.290990 00	1.696300 00	9.338400-03
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Figure 85. (Continued)

2.08246D 05 6.69139D 04 0.7346D 05 6.1393D 04 2.4709D 03 1.52115D 03 1.82115D 03 1.8154D 03 1.82115D 03	7.05350 04 -2.469710 04 -2.469710 05 -5.471200 03 -6.03420 02 -6.03420 03 -6.60660 03 -6.60660 03 -6.60660 03 -7.0060 04 -7.0090 04 -1.97090 04	386900 132690 132690 132690 132690 136990 136990 136990 136990 136990 136990 136990 136990 136990 136990 136990 136990 136990	7.5.28.28.20.03 4.5.25.95.00 2.6.6.26.00 2.6.6.49.00 2.7.98.72.03 1.176.72.03 6.926.46.00 6.926.46.00 6.926.46.00 7.2.738.20.03 2.16.70.03	2.15312-0.3 2.153120-0.4 2.153120-0.4 2.34686-0.2 2.47780-0.4 3.47780-0.3 2.204070-0.5 2.204070-0.5 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.34770-0.3 2.
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Figure 86. (Continued).

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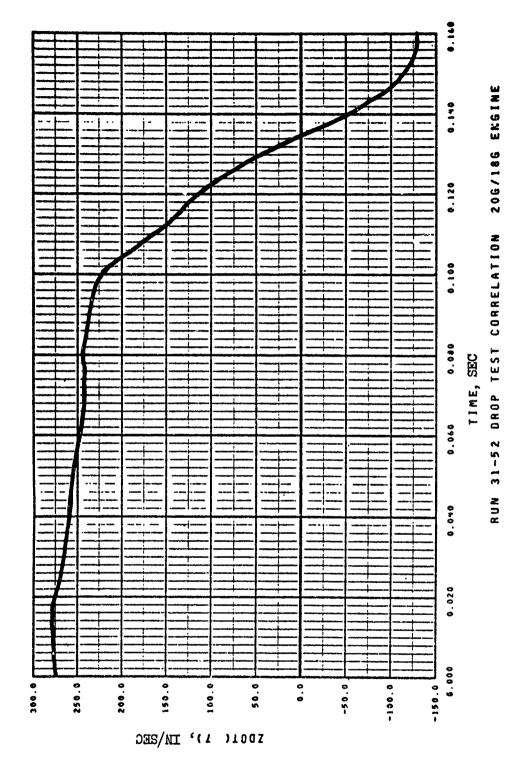


Figure 87. Engine Mass Vertical Velocity Time History Plot, Combined Vertical and Lateral Impact Sample Case.

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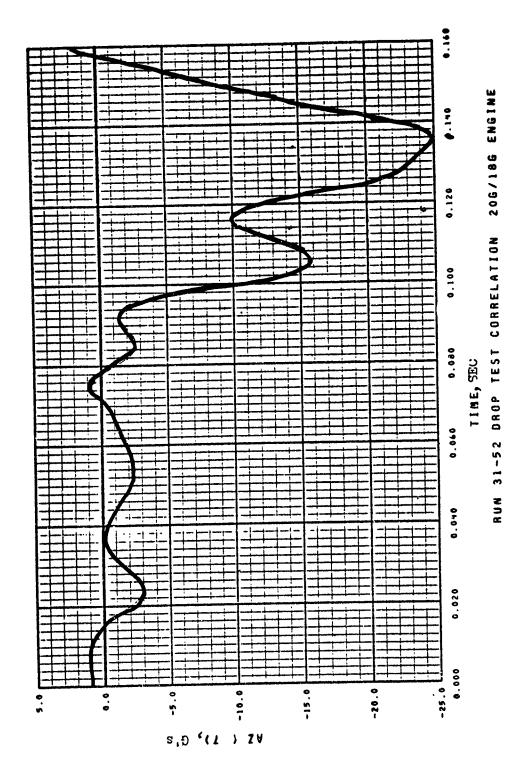
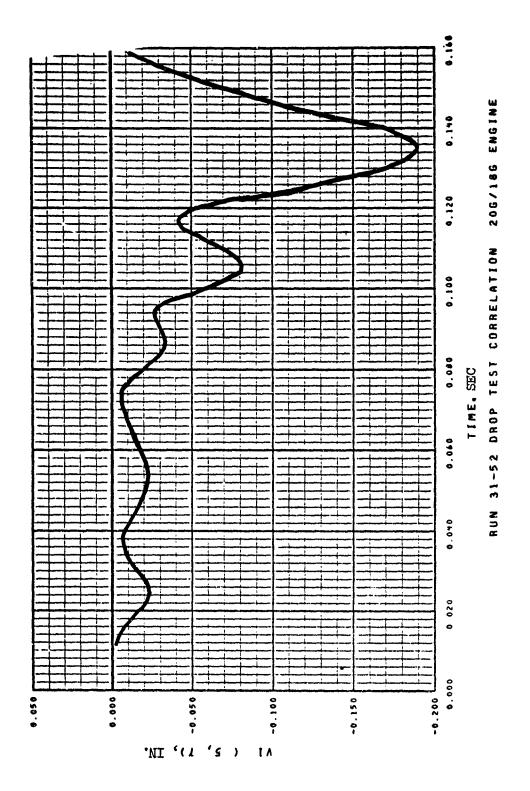


Figure 88. Engine Mass Vertical Acceleration Time History Plot, Combined Vertical and Lateral Impact Sample Case.

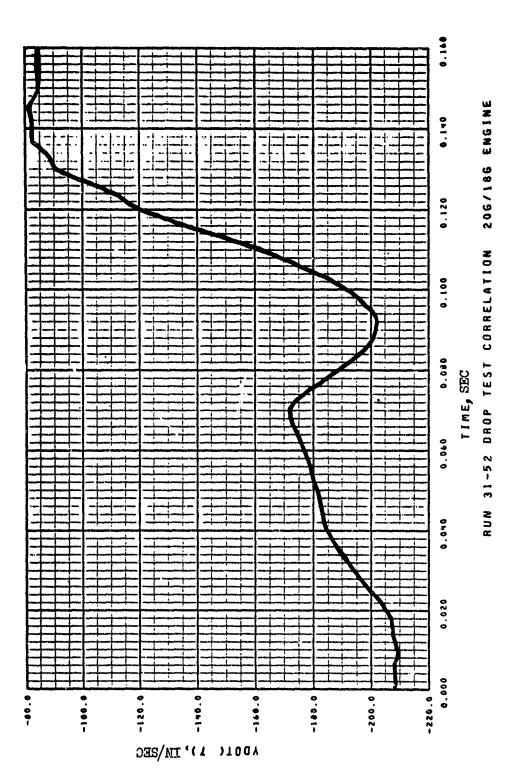


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Figure 89. Engine Mount Vertical Deflection Time History Plot, Combined Vertical and Lateral Impact Sample Case.

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Figure 90. Engine Mass Lateral Velocity Time History Plot, Combined Vertical and Lateral Impact Sample Case.

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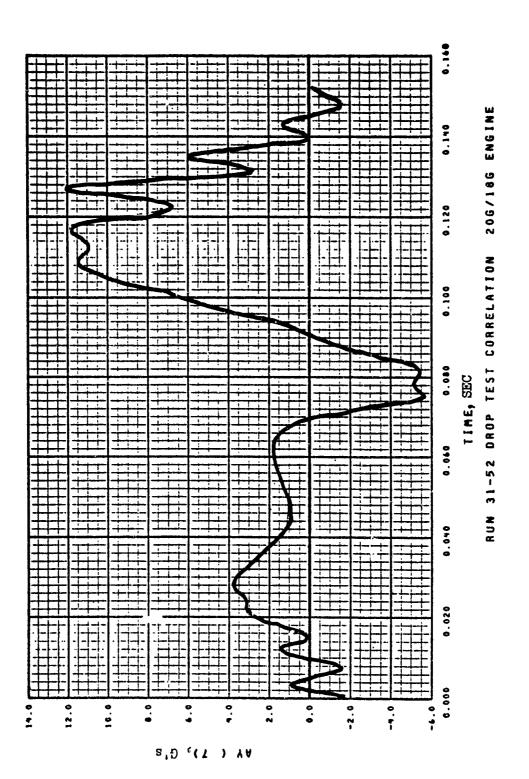
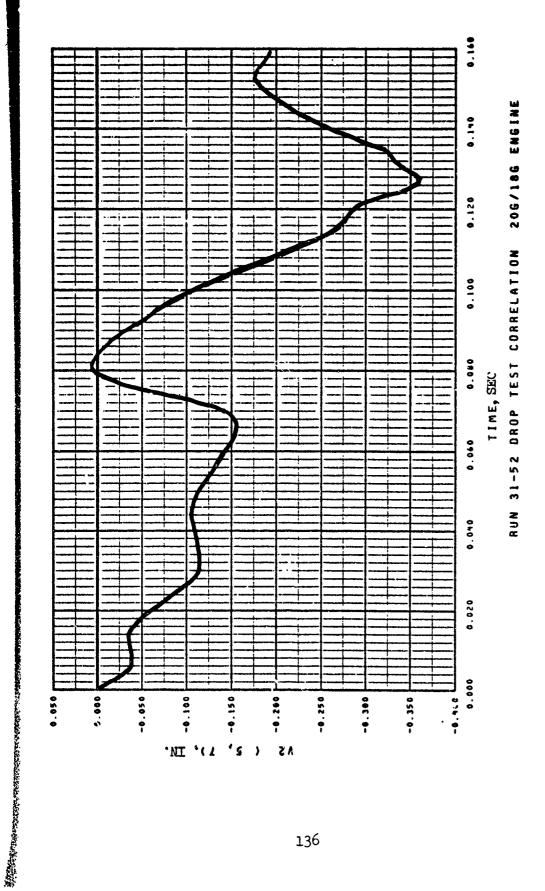


Figure 91. Engine Mass Lateral Acceleration Time History Plot, Combined Vertical and Lateral Impact Sample Case.

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Engine Mount Lateral Deflection Time History Flot, Combined Vertical and Lateral Impact Sample Case. Figure 92.

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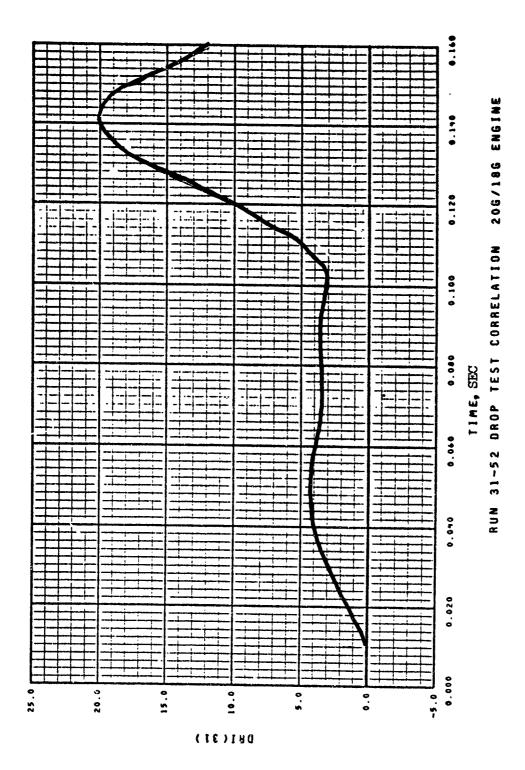


Figure 93. Forward DRI Time History Plot, Combined Vertical and Lateral Impact Sample Case.

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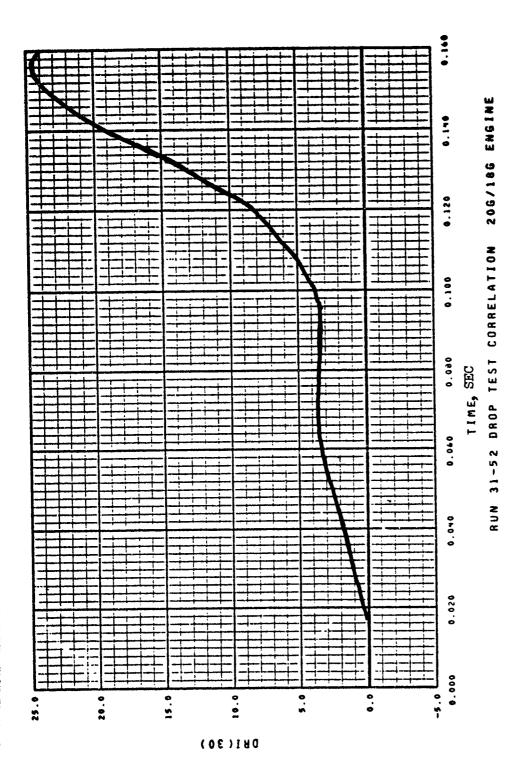


Figure 94. Aft DRI Time History Plot, Combined Vertical and Lateral Impact Sample Case.

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Figure 95. Input Data, Combined Vertical, Lateral and Longitudinal Impact Sample Case.

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ļ		-3.512770 03	010131			100000000000000000000000000000000000000	10-001595	10-00700407
	•		.012700	88	-2.447180 01	1	00 0000000	
MASS	7	1	.163480	2	1-	-2.64626D-02	-1.847090-03	3.754710-03
	~ ~	2.47038D 02	.741180	20	-3-397900 01	-6.001630-01	-2.478090-01	-3.296300-01
!			234770	20		732619 61	-4.390000-01	10-01/096-6-
	Ť	860130 00		8	-2.965760 01	10 01075	70 0000000	71006
MASS	3.	326430		10	•	-2.316350-02	-5.404550-02	-7.325026-03
	4.	3-149710 02	-2.046640	26			-1.929680 00	4.059980-01
!		70 016701 6	2000000	7.5	10 00000-1-	10-003 700-01	-1.91578D_00	10-086869
	•		77477		- 10 DC 70 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	3	10 004104011	10 062/66-1-

Figure 96. Typical Output, Combined Vertical, Lateral and Longitudinal Impact Sample Case.

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	X00X	100 >	2001 TO 74	T1001	THETABOT	100154
	VOOT	VOOT	ZACCEL	1004	1000	RDOT
MASS 0				-2.17793D-02	-1.305530-02	2.154460-04
			-1.232710 01	-6 - 101 9 -0 -01	-4.704870-01	-2.682480-01
				-0-019e61-01	-6.62778D-01	-2.867450-01
	-8.075250 03	2.109725 03 5.236475 00	-9.760600 03 -2.447920 01	-1.316430 01	-1.755510 01	-2.178370 01
MASS 10	3.838520 06	-2.002530 01	-1.179550 01	-2.427435-02	-5-937590-02	-4.410530-03
				-5.489390-01	-1.854930 00	-3-819760-01
		.39377D		-5.716260-01	-1.544730 00	-4.262130-01
	-3.49564D 03 -6.972f 30 00	2.449330 03 6.544350 CO	-6.041380 03	1.516740 01	8.10367D 01	1.102760 01
4455 11	2.58R35D 01		-1.10075D 01	-2.427840-02	-1.054170-02	1.702580-03
	2,235340 02	-1.476470 C2		-6.881470-01	-6.745840-01	-1.750500-01
		470220		4.899930-01		-113450-01
	-3.333560 03	2.65674D 03 6.73917D 00	-9.707520 03 -2.449880 01	-2.505020 01	2.912140 01	-1.556550 01
EXCC 15	6. 4278.70 DI	2.400000 01	** *** *******************************	-9 wei 150-05	-2 280440-03	40-041101 4-
				20 01100	20 - 20 - 20 - 20 - 20 - 20 - 20 - 20 -	
			-1.904570 01	1 - /27130 00	-3.224580-01	-1.046020-01
		1.540520 03		-3.282150 01	-3.329300 01	7.507000 01
	-1.249850 01	3.940040 00	-2-387370 01			•
MASS 13	9.53495D ni	-6.49840D 01	-1.256250 01	₩.716330-03	-2.82557D-02	4.051550-04
				7.657660-01	-6.744470-01	-7-259400-01
				7.452560-01	-8.699730-01	-7.315270-01
	-1.257610 01	1.413050 03 3.375970 CO	-1.270400 04	1.131230 02	-2.257850 01	-3.346760 01
MASS 14	1.045600-01	4.233390 01	-4.44195D OO	-7.44800D-03	-1.36549D-C1	1.404430-01
		-7.564720 01	-8.247360 01	9.688910 00	-1.753590 00	-2.459480-01
	2, 795630 02	-1.170650 02	-1.23132D 02	9,854540 00	-1-751730 00	-2.566 990-01
	1.301070-01	-7.3963[0-03	10-00#50#·# 00#50#·#	10-001624-1	10-008067*8	0.0
MASS 15	4.680220-01	-6.796725 01	-7.974130 00	8.565930-02	-1.243250-01	-1.344580-01
	2-842070 02		-1.549410 02	00 019856.9	-1.707130 00	-1.114470 00
	3.214200 02	20 046521.5	1.650590 02	-7.07661D 00		-9.557120-01
	1.240050-01	1.511.510 US 8.489430-02	10-0E+980.6	10-061650.6	00 011227-7-	•
MASS 16		025866	-1.261440 01	-1.499330-02	2.629410-03	1.806230-03
	2,261530 02	-1-604740 02	-3-54 780 01	-2.10-010-01	00 015450-1-	10-026676.1-
	-3.070180 03	1.488960 03		8.18602D 01	-2.020510 01	-1.090700
	-7.422440 00	74.76				

Figure 96. (Continued).

RUM SE-31-100 COMBINED VEL. IMPACT TIME • 0.10000

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i	X X D	1007	2007 W	PHIDOT	THETADOT	PSIDOT
l	U00T XACCEL	VOOT	MOOT	P 004	1000	RDOT
HASS 17	9.07% 10 01 2.758320 02 2.538950 02 -1.977110 02 5.723410-02	4.21921D 01 -7.07926D 01 -8.10447D C1 -6.65489D 02 -3.35262D-01	-3.925540 00 -7.751980 01 -1.267810 02 2.6549C0 02 9.403850-01	-3.424470-01 1.457860 00 1.547060 00 -4.734150-91	-5.72654D-02 -3.77241D-01 -8.77836D-01 -1.18679D 00	1.79543D-01 1.55849D 00 1.33690D 00 -1.48030D-14
91 3 5 M	9.26122D 61 2.811696 02 3.240795 02 4.231330 02 4.75v930-02	-8.41217D 01 -3.39929D 02 -3.10482D 02 5.41999D 02 3.54798D-01	-4.30356D 00 -8.34528D 01 1.20803D 01 -8.77711D 01 9.33731D-01	3.631290-01 -6.012290-01 -6.705520-01 5.409120-01	-4.761730-02 -2.394360-01 -7.465490-01 -4.897850-01	-1.486460-01 -1.456400 -1.274830 00 -1.480300-16
MASS 19	1,439320 02 2,194150 02 2,190930 02 -2,990810 03 -7,818970 00	-1.989255 01 -1.675610 02 -1.680865 02 9.466110 02 2.375070 00	-1.286090 01 6.817230 00 3.686110 00 -7.514200 03 -1.678260 01	-1.57985D-02 -4.46457D-01 -4.46451D-01 1.07226D 01	4.429600-05 -7.118480-01 -7.092100-01	1.944670-03 -1,336440-01 -1,470016-01 -1,109890 01
#ASS 20	8.572190 00 5.090570 02 4.590310 02 7.077550 03 2.029030 01	2.351700 01 -1.794130 02 -1.834650 02 2.313510 03 6.926610 00	-6.635600 01 -4.351070 01 -7.664330 01 -1.174470 04 -2.225770 01	-2.83264D-02 -7.66501D-01 -6.64757D-01 1.797735 01	-9.907309-02 -6.038080 00 -6.064640 00 -2.246830 02	1.333570-02 1.028230 00 9.517645-01 -6.672960 00
MASS 21 MASS 22	\$.346010 00 \$.464140 02 \$.464140 02 1.001570 04 2.785000 01 3.137030 00 2.231550 02 2.231550 03 4.250400 03	-6-632250 01 -1-647780 02 -1-647780 02 7-993010 02 3-19410 00 2-49460 01 -2-437910 02 2-453630 03	-6.414730 01 2.481730 01 2.425110 01 -1.302690 04 -3.079920 01 -1.272670 01 -6.09320 01 -6.09320 01 -7.270970 03	-2.217940-02 -1.02459 00 -3.559320-01 -3.85870 01 -2.84420-02 -1.39480-01 -7.318950-01	-1.39866-01 -6.54780 00 -2.648300 02 -4.649080-02 -4.649080-02 -4.649080 00	1.059940-02 1.209590 00 4.1007430 07 4.1007430 07 1.477876-02 2.399520-01 1.002040-01
MASS 23	1 -	2517550 2517550 2517550 2517550 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 251750 25175	t l.	-1.4.79790-02 -5.724160-02 -3.137790-01 -2.031270-02 -2.019890 60 -2.019890 60	-1.316370-01 -1.210500 00 -7.210500 00 -2.395240 02 -2.229180-02 -2.229180-02 -2.239880-01	-4.104.180-02 -2.104.190-02 -2.104.190-03 -2.104.190-03 1.074.480-03 1.074.480-03 1.074.190-03

Figure 96. (Continued).

Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot Vot		*	^	•	\$77 8	44374	
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VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL VACCEL V		Ð	>	=	•	0	2
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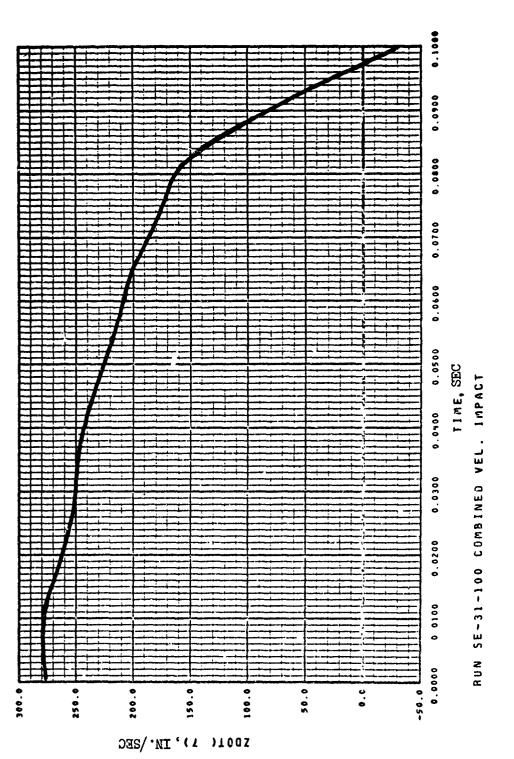
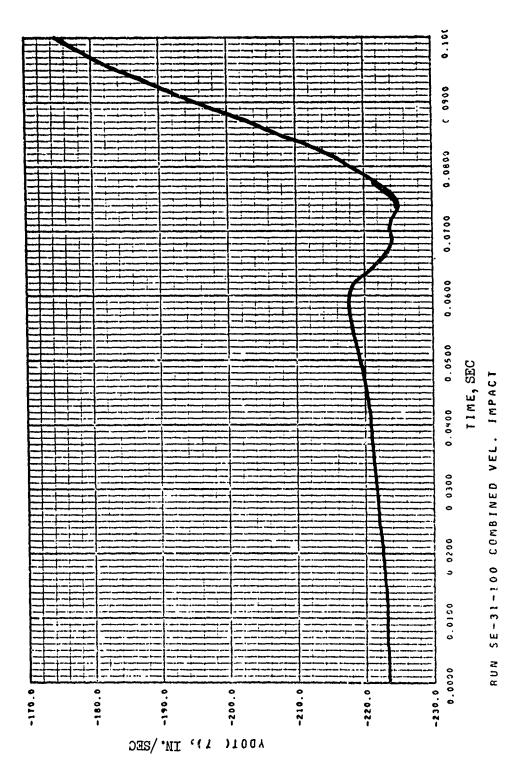


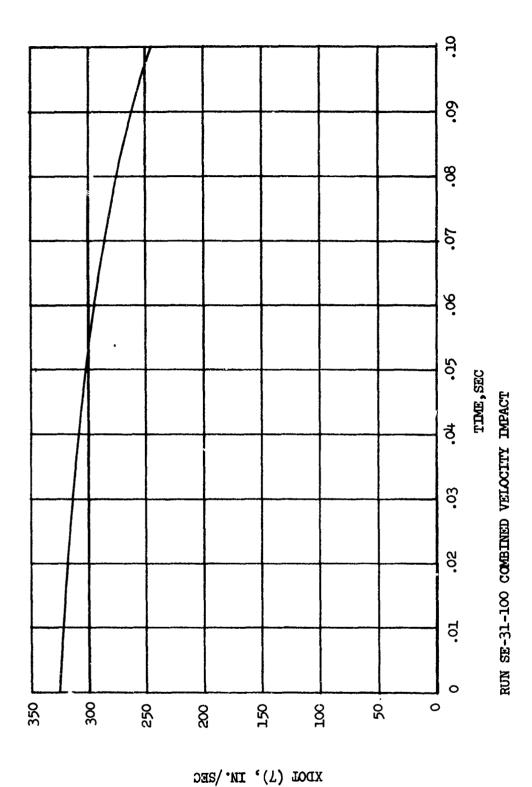
Figure 97. Engine Mass Vertical Velocity, Combined Vertical, Lateral and Longitudinal Impact Sample Case.



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Figure 98. Engine Mass Lateral Velocity, Combined Vertical, Lateral and Longitudinal Impact Sample Case.

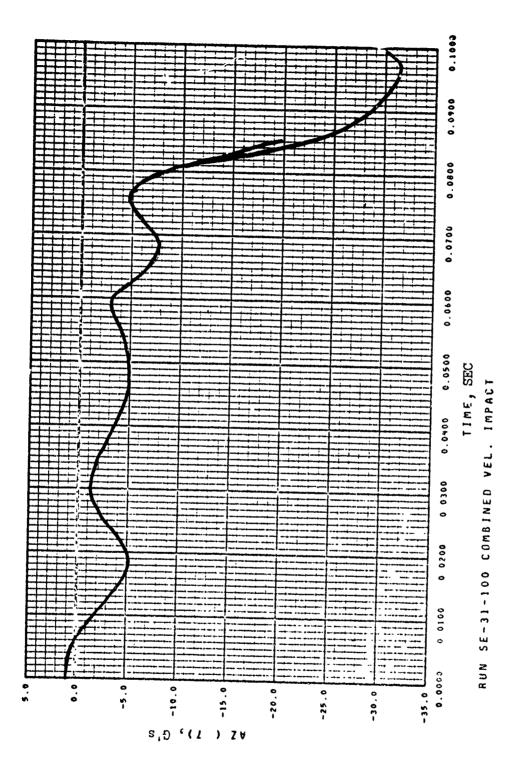
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Figure 99. Engine Mass Longitudinal Velocity, Combined Vertical, Lateral and Longitudinal Impact Sample Case.

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Engine Mass Vertical Acceleration, Combined Vertical, Lateral and Longitudinal Impact Sample Case. Figure 100.

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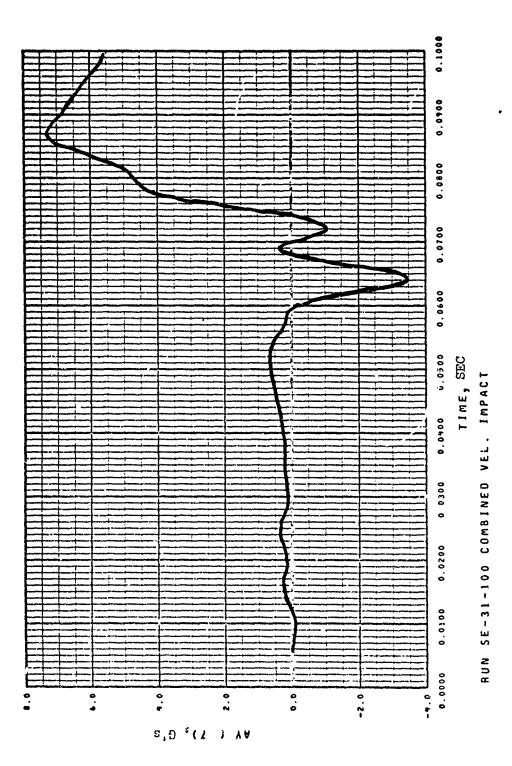
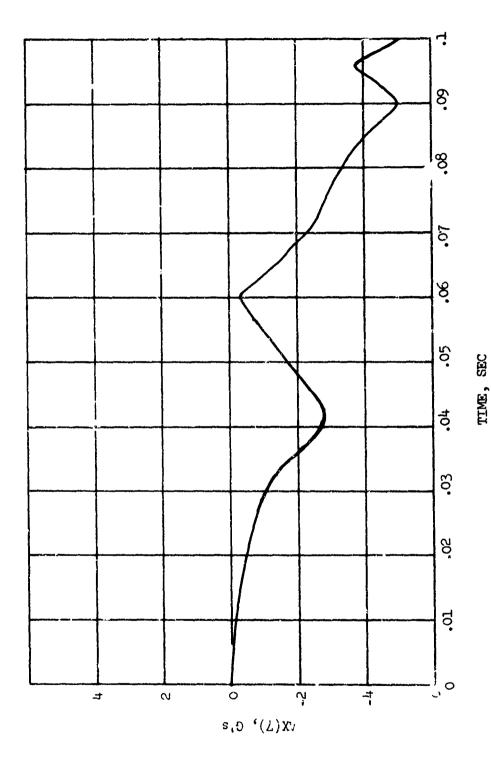


Figure 101. Engine Mass Lateral Acceleration, Combined Vertical, Lateral and Longitudinal Impact Sample Case.

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Engine Mass Longitudinal Acceleration, Combined Vertical, Lateral and Longitudinal Impact Sample Case. Figure 102.

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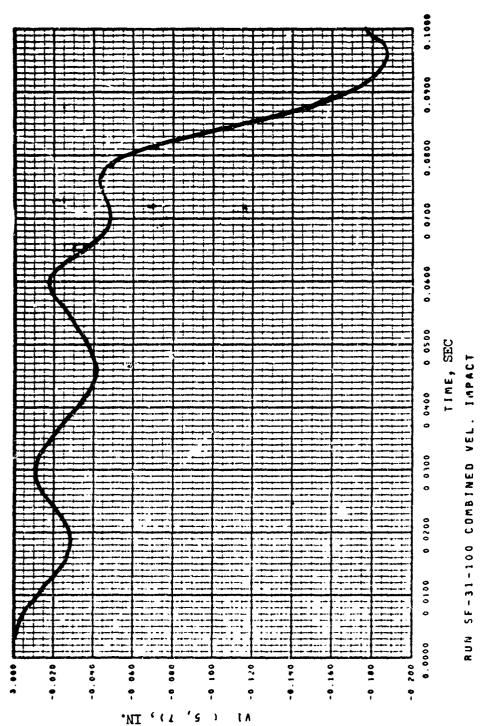
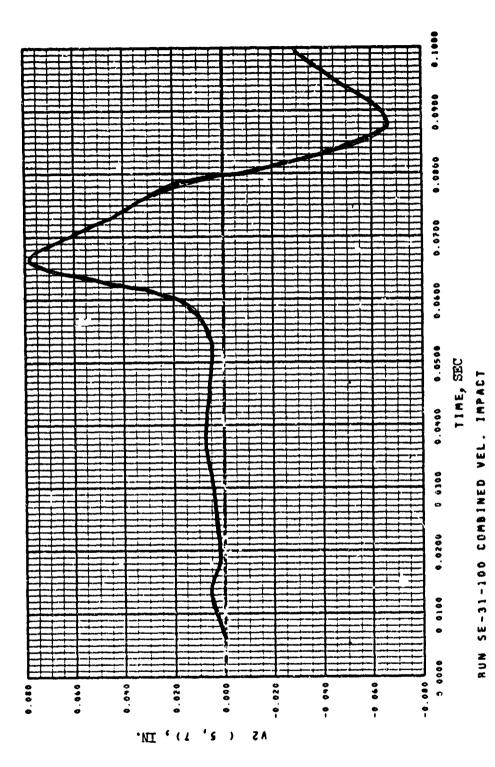


Figure 103. Engine Mount Vertical Peflection, Combined Vertical, Lateral and Longitudinal Impact Sample Case.

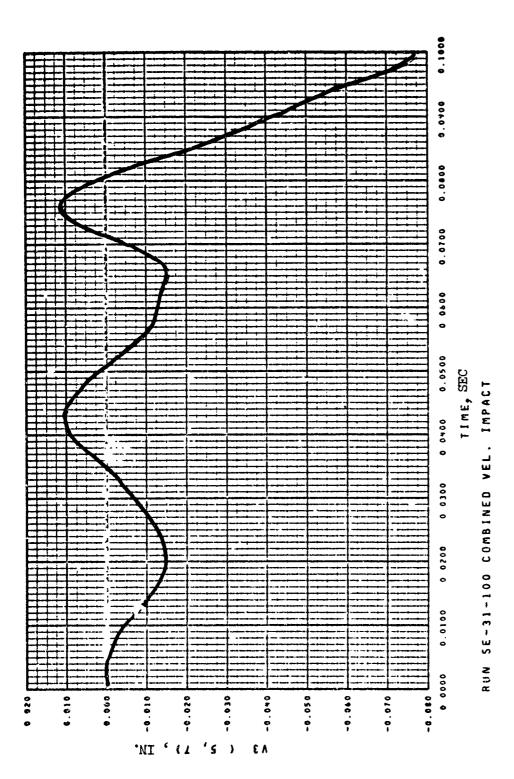


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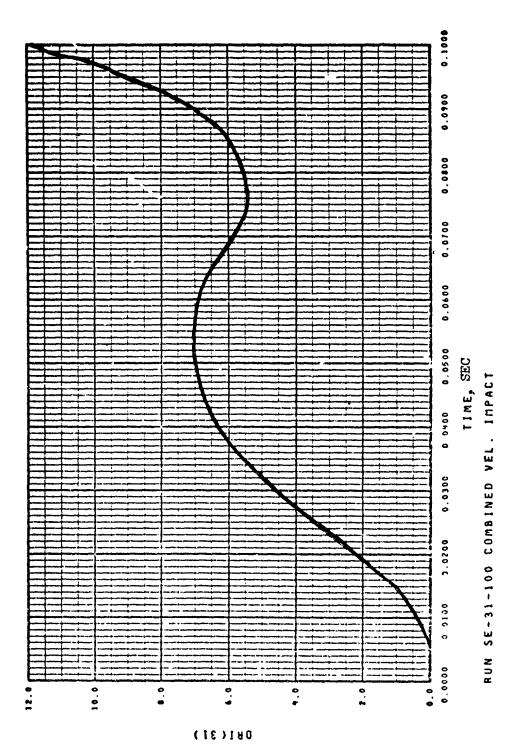
Figure 104. Engine Mount Lateral Deflection, Combined Vertical, Lateral and Longitudinal Impact Sample Case.

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Engine Mount Longitudinal Deflection, Combined Vertical, Lateral and Longitudinal Impact Sample Case. Figure 105.



Forward DRI, Combined Vertical, Lateral and Longitudinal Impact Sample Case. Figure 106.

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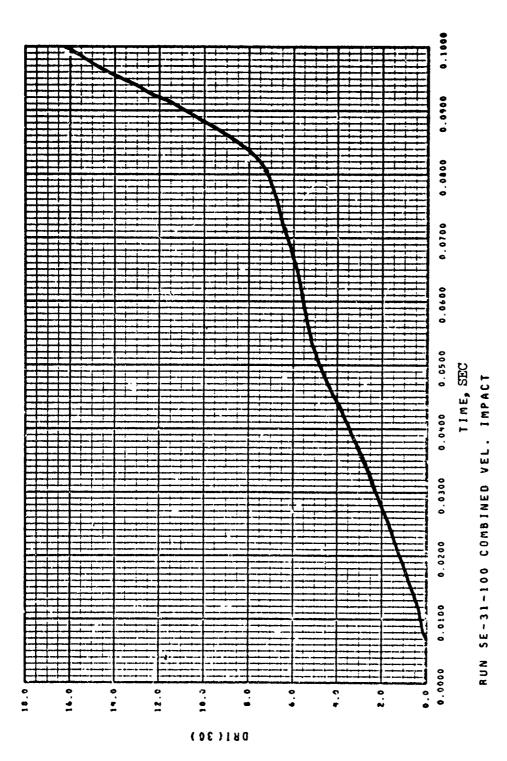


Figure 107. Aft DRI, Combined Vertical, Lateral and Longitudinal Impact Sample Case.

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Figure 108. Program Listing.

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Figure 108. (Continued)

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		C# 4 C / E

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15N 01.4	-	CAS01470
15N 6175	x(1) = x(1)+px(1)	094[0. 7
3710 MSI	_	CKSO1440
154 0151		C#S01500
148 017	7211 - 711141711	CRSOLISIO
158 6130		CR501520
15H 0131	TATION # PER 120-120-120-120-120-120-120-120-120-120-	000000
ISN 0137	UTHETALL) = DELITATOTHE COTETT	C#501450
25N 0133	THETALL) = THETALLD+DTHETALLD	CRS01360
15N 0134	UPSICED = DELTATOPSICOTOR	CRS01570
ISM OLDS		CRS01560
of 10 WS1	200 CUNTINUE	CR501590
TOTAL STATE	190 TIME - TIME+DELIAT	CRS01600
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5	IPLC + IPLC+1	00000000
6	JELIPLC. EU. JIPLOT) CALL SAVE	
		0691050
15N 014B	280 DU 310 I = 10NM	C3501:00
ISR CLAY	1 = PINU(I)+012*P(I)	01210283
1SN 0150	FING(1) = PIN(1)	CESCIANO
15h C151	FINE T	CR501740
15N 0152	UPINITY - PINITY-PINULIA	CKS01740
15W 01:3	T = UINU(1)+01,+0(1)	CRSC1750
4510 HSI	CINC(1) = CINC(1)	CR501760
2010 801		CR501770
10 015		CRSU1760
15k 015k	C120111017111	CA SO1740
154 0154		CR301860
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JSM 0103	1 = XULU(1)+U12+XUUT(1)	07310743
15N 0162	AULU(I) = X(I)	CRSC1740
15% C163	X(1) * 1	CRS01#50
4315 MSI	באכון - אנון-אנונטנון	CRS0186C
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24.0		CASOLEGO
ISM CLUB	(141) x Y(1)-Y(1(1)	CRS01890
15% 0169	1 = 20L0(1)+012-2001(1)	CRACATO
15W 0170	ZULC(1) = 2(1)	
1 SN 01 71	1 = (11)	
15H 0172	DEC11 - 2(11-20LD(1)	CRS01940
210 451	PATOLOGIA - PATOLOGICA	CRS01950
150 0176	THEOLOGICAL STREET	CR501960
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Figure 108. (Continued)

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				JPS1(1) = PS1(1) + S1U(0(1)	CR 562060
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				1 1 1 1	CR562120
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1 0337	X1(1) = X1(1)+DXL	CR506160
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Figure 108. (Continued)

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1300 = 13000 200 = 10000 VELUT(130K) = VAD-5UM	CR513440 CR513500
	CRS13510 CRS13520 CRS13530
UC 105 K ± 1,3 1F(15FKF) 10+105+110 110 1F(VC.4KF) 105+105+115	CRS13540 CRS13550 CRS13560
5K = 5(K) 5(D) = 5(K-5(1) x) 14 (1) x = 5(K-5(1) x) 14 (1) x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10 x = 10	CRS13570 CRS13560 CRS13560
1F(11.5f(+K)) 130+130+136 1B5(1K) = 1 1B5(1K) R513600 CR513610 CR513610	
15 (15 St.) 140-150-150 F550 = 0.0 6.11 190	CRS13630 CRS13646 CRS13650
= SR-F5FBAR(1,K) = SLAR-F5FBAR(E (SF) 150-155-155	LRS136h0 (RS13670 (RS136#0
50 FSFU = 0.0 fu To 1oc CDMPUTE FSFO PER MEW EXTERNAL SPRING LOAD-STROKE CURVES 7/25/72	CRS13890 CRS13700 CRS13710
50 TG 160 GU TG 157	CKS13720 CRS13730 CRS13740
45 TU 160	CRS13750 CRS13760 CRS13770
6.0 10 16.0 15.7 ESPOLITE/SPOLITE/NI)+(SP-SA(1,K))*(FSPO-FSPOLITE/K))/(SB(1,K)-SA(1,K)) 80 IF(SK(1,K)) 165,165,170	CRS13780 CRS13740 CRS13600
176 MMilati . 0 60 10 10 190 165 18fMMilati 190,175,190	LKS13&10 CRS13&20 CRS13&30
MMOISK) = 1 	CRS13840 CRS13850 CRS13860
	CRS13670 CRS13680 CRS13890

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Figure 108. (Continued)

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15K 0600	ELUIVALENCE (PPK, XMPR(1)), (OPR, XMPF(2)), (KFK, XMPR(3)),	CPS14260
	1 (PHIEPR, ANGEPRILL), ITHEOPR, ANGOPRIZIT, (PSIUPR, ANGOPRIZIT)	CRS14270
1090 851	0.0	CKSIAZEC
128 COC.	WALL # 0 0707 D2	CKS14250
1000 MV		00071040
15M 0605		CE41430
154 0606	(o a di) //	01671747
15N 0607	10 2020 1 # 1 * NE	CRS 44360
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15N 0604	(1) 6336 (1) (1) (1) (1) (1) (1) (1) (1) (1) (1)	CK\$14.160
15M 0610	2020 2010 = 2010 + 4010 1 1 1 1 1 1 1 1 1	Ch514370
	XCLF = XLDP/WIOT	CR514300
15N 0612	Y509 # Y507/2451	Ch514340
	1014/4027 = 4097	Ch51-400
	C APRIME AND ABANDINE (3)	CK514410
15N 0614	CALL LULEK(APR.PHIPR.THLPR.PSIPR)	CRS14420
15N 0615		CK\$14430
15N OC 16	C1 = C0:1743743	CR514440
15N 0617	SE ATTAINED	CR514450
15N 0c18	C2 x CIS(TPEPH)	CH514460
	C NOW ALKRYIME (4)	CR514470
15N 0614	Abakrt1.13 H 1.0	CR514460
	4	CKS14450
	H	CR514500
15N 0622	*	CKS14510
15h C623	Ľ	CK514570
15K 0624	ABARPH 1.5-21 = 51/C2	CKS14530
15N 06.5	AEAFFK[1,3] = C1*52/C2	CRS14540
15K 0626		CK\$145.50
15h 06.27	AEAKFH(3,2) = C1/C2	CK\$145&0
	C ANGLE DUT PRIMES (6)	CKS14570
1SN COZH	CALL MATVECIABAPPROXMPROANGUPROD	CRS14580
	C D PRIME (7)	CR514540
15N C624		CR51+600
15M 06 30	_;	CK514610
15N 0633		CKS14620
ISN 0632		CRS14630
1SN 0633	*	CK514640
1SN 0634		CR514650
ISN 0635	UPR(3,1) = -DFR(1,3)	CAS1466G
15N 0636	UPK(3.2) = -[PK(2.3)	CR514670
15N 0657		CRS14660
	C A DOT PRIME (%)	CR514690
13N 0638	CALL MAIMUL(APR.DPR.)	CKS14700
1SN 0639	ZCMAX # U+O	CKS14710
	V 4001 3	CK514720
15% 0640	00 2040 I = 1,™	CR\$14730
	C AI DOUBLE PRIME (9)	CRS14740
15N 0641	CALL EULER(AIDP.PHIDP(I),THEDP(I),PSIDP(I))	CRS14750
	. 41 5101	67675765

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SN Ub42	CALL HAIMULIAPK, AIL P, AIC)	CKS 24,770
SN 0643	FFETALLY = -AKSIN(AIL(3.1))	CR514760
15N 0644	CT + 1.C/CUS(THETA(1))	04241340
15N 0645	FH111) = AFSIN(A1C13,2)=CT)	CR\$14#00
15N 0646	PSI(1) = AKSIN(AIC(2,1)0C1)	CR214830
	C (14.)	(KSI4R, U
7490 MS		CR514830
	1114001 11241 1127 1127 1127 1127 1127 1127 11	
100 001	1 101 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	00011100
Or Or Wall	# 1	0001000
	16(1:1) 2066-2050-2060	CKS14680
SN 0652		CKS1419C
SN 0653	DO 4070 L # 1.3	CR514900
35N 0654	2070 VC = VC+APR(3,13+VJP(3,1)	CR\$14910
15N 0655	1+ (VC-2CMAX) 2050-2050-2080	Ch. 514720
15N 0656	2080 2CMAX = VC	CRS1493C
15N 0657	2050 CENTINUL	CRS1454C
	C ENG OF LEGIF A	CR51-950
ISN 0656	2040 CUNTINIE	CK514560
15% 0659	16(26) 2220,2210,4220	CR514970
15N 0660	210 (L = -(CMAX((1)0	CR514960
	C SEE IF PHISTS ETC. ARE ALL ZERO	CRS14490
ISN OF 61	2240 LOCATIO 1 = 1.0MM	CR5150C0
SN 0662	15(05)(19(13) 2150-2110-2150	0,051503
15N 0663	2116 1F (1H) DF (11) 2150,2120,2150	CR\$15020
15N 0664	212(IF(PSICH(1)) 215C,2100,2150	CK515030
15N 0665	\$100 CON11nut	CKS15040
	C IF WE CEI PLIKE ME COMPUTE NEW THETACIOUS AND PSICIOUS	CR515050
ISN COCC	PI = 3.1415426535897432400	CRSISCRO
1000 NSI	14*00¢* + 714	CRSISOR
3 2 M Ober 8	10 11 11 11 11 11 11 11 11 11 11 11 11 1	CKSISCEO
100 NO.	771	CR: 130 V
000 40		C1515100
100 200		CKSISTIC
20.00	1741 1741 1741 1741 1741 1741 1741 1741	031770
124 OF 12	**************************************	05151545
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ISN GE 75	2180 PS11J(1J) = 0.0	Ch515160
15N C677	TELLICIAL ATANZIZIANA	CR515170
15N 0676	CU 18 2260	CK\$15160
61 90 NSI	7176 PSILD(T3) = 6.0	CRS15150
15% 0680	THE LOCAL STATES	CK\$15200
	1F(Z1JP) 21c0.2200.7200	CR515210
ISM UCES	ZIOD 1ME 33(13) = F12	CR\$15220
15N C663	60 10 2260	CRS152.0
ISR CORP	SIAO PSIIJ(IJ) = AIARZ(VIJP)	C#515240
ISM Cous	THEIJ(IJ) = -AIANZ(ZIJP,SURT(XIJP&XIJP&YIJP&YIJP))	CKS15250
15M 0666	2200 CONTINUE	CRS15260
	2,0001	CR\$1\$270
ISM CORT	2150 UO 2050 1 = 1+MM	CR515280
ISM CARR	VIPELL H VOPERALL	CRSISSO
97.50		

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3593 WA	(E.1)45 = (E)414	CKS18910
		07507
158 G691	CALL MATVEC (APR.VIP.XV.O)	CRS15330
2690 NS	-	CKS15340
TON COOP	_	(4515350
15M 00%	- 1	CKS15360
INE CEAN		CK515370
	C (15)	CK\$153E0
15M 06%6	CALL MAIVICIANTHONE, XV-01	CRS15340
2490 HS		CR513+00
SN 0658		CK515410
SN 0649		CRS15420
SM 0700		CRS15430
ISK O701	Y0U1(1) = XV(2)	C#812440
SM 0702	(E) Xx = (I) 1002	CRS15450
	(16)	CRS15460
SN 0703	CALL MATVLC(AIC,XV,V2P,1)	CRS15470
SN 0704	U(I) = VIP(I)	CR51548U
15M 0705	V(1) = VIP(2)	CK\$1549U
SR 0706	ECEN E VIITURE	CAS15500
	C (17)	CK\$15510
ISN CACA	CALL MATVEC(ALUP,XMPR,VIP+1)	CRS155.0
SR 6708	P(1) = V1F(1)	CRS15530
15# G7C9	U(1) = VIP(2)	CK\$15540
15N 071C	R(1) = VIP(3)	CR\$1555
	C AIBAR (16)	CRS13560
SM 0711	S	CRS15570
15N 0712	C1 = COS(PHI(1))	CP.515580
ISN 0713	*	CRS15590
15N 0714	C2 = CC5(1HE1A(1))	CR\$15600
15N 0715	AbAkr(1,2) = 51e52/C2	CKS1561U
SN 0716	ABAPPK(2,2) = Cl	CR515620
15N O7		CR515620
SN 0716	A5AFFF(1,3) = C1*52/C2	CRS15640
SR 0714	Abarpk(2,3) = -51	CRS15650
15N U720	ABARPK(3.3) = C1/C2	CR515660
	(41) 2	CRS15670
SN 0721		CRS156e0
SN 0722		CRS15690
15N 0723	*	CAS15700
SN 0724	PSIDOT(I) = XV(3)	CRS15710
	C END LOUP	CRS15720
57 0 NS	2040 CDV1140E	CRS15730
9210 MS1	TO TO TO TO TO TO TO TO TO TO TO TO TO T	CRS15740
270 MS	Z301 FORMAT(1HO, 113, THE 13(13), PSIL13(13)")	CR515750
8240 NS	PRINT 2300+(1J-1HE1J(1J)+PSIIJ(1J)+IJ=1+165)	CRS15760
	2300 FOKMAT (1H +15+1P2£15+5)	CRS15770
SN 0730	REJUKN	CRS15760
		CR515750
		CR515800
15W 0731	12/14 YAZIN	CRSISBIO
		CRS15820
***************************************		CRS15830
3010 20	11 AL 11 OU 14 FE 1 OU 1	

CRS13640 CRS15850 CRS13660	CR515670	CR515890	CKS15900	CRS15910	CR\$15920	CRS15930	CK215440	CRS15460	CR515970	C#\$15960	CKS15590	CR516010	CRS16620	CRS16030	ZRS16050	C#S16060	CR516060	CRS16090	CR516100	C#516110	CRS16130	CRSIBILG	CR\$16150	CRS16170	CRSIGIEO	CR\$16190	CRSIBZOU	CK516210	CK 516230	CR \$ 16240	CRS16250	C# 516270	CRS16260	CR\$16290	CR 516300	CESTO210	CR516320	CRS16340	CR516350
UJALNSJUM VELZIG-BUJ EGUIVALPMCE (VEEII),VEEZIJ) ILINES - 00	9 = 161	C FORCE NEW YARD	MFK 2 1000	DD 5059 3 × 1-NM	IF (ILINES-NPK-IPL) 3010-3020-3020	SOURCE AND STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SOURCE STORY OF THE SO	UNIVERSITY AVENUES	3200 FURNATITH 6411HE F9-5-/	PRINT 5300	PKIN1 406		77 - 1000 1000 1000	3300 FURMAY(1H 916X-1HX-14X-1HY-14X-1HZ-13X-3HPHI-11X-5HIHETA-	11X33FFSI) - TOURHETCIH - 17X-4HXCOI-11X-4HYDOI-11X-4HZDOIT-10X-4HDMIDGI	1 bashit. 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LOX. CHXACCEL, CX. CHYACCEL, CX. GHZACCEL./)	MOK K III C	SOLD FFF & MFK4JFT.	FINE 600- X001(1)-V001(1)-V001(1)-FH10G1(1)-FINEDG1(1)-FS1001(1)	FRIAT 600, U(1),V(1),M(1),P(1),Q(1),R(1)	PRINT BOC, UDCHILL, WDCHILL, WDCHILL, PDCHILL, GDCHILL, RDCHILL MAINT BOC, WASCELL, WASCELL, PACCELL	PRINT BEO	700 FUKMA1(1M .5MMASS .12.2X,196E15.5)	BOO FORMAILLM .5x.1P6E15.5)	3099 CUN 1 YOU	OF A 11 CARDINAL CARD STATE OF A 11 CARD CARD CARD CARD CARD CARD CARD CARD	* *SUMDF:4-1J1 .SUPUF(5-1J1) .SUMOF(6-1J1")	PRINT BIO, (IC(15), JG(12), (SUMDF(K, 12), KEL, 6), I JEI, IGS)	PRINT 1 (5) (1) (1) (1) (1) (1) (1) (1) (1) (1) (1	11.1.VE. 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Figure 108. (Continued)

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Figure 108. (Continued)

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CANTILLER OFFIUNS - NAME: MAIN, OFFIED2, LIME(NT=56,512E=DODON, SURVELEDCY, NAME: MAIN, OFFIED2, LIME(NT=56,512E=DODON, SURVELEDCY, NAME, NAME (MAIN, NAME) MAIN, NAME (MAIN, NAME) MAIN, NAME (MAIN, NAME) MAIN, NAME (MAIN, NAME) MAIN, NAME (MAIN, NAME) MAIN, NAME (MAIN, NAME) MAIN, NAME (MAIN, NAME) MAIN, NAME (MAIN, NAME) MAIN, NAME (MAIN, NAME) MAIN, NAME (MAIN, NAME) MAIN, NAME (MAIN, NAME) MAIN, NAME (MAIN, NAME) MAIN, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, NAME, 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UMPILER OFTIUMS - MANKE MAIN, UPIEO2, LINECNIT-56, SIZE=DODOOK, SUMKLE, BCUP, MULIST, WOLKEL, BCUP, MULIST, WOLKEL, BCO, SIZE = DODOOK, SUMKLE, BCUP, BCO, SUMKLE, BCO, BCO, BCO, BCO, BCO, BCO, BCO, BCO	UMPILER OPTIUNS - NAME	### CAN TO PERSONAL TO PERSONAL PROPERSONAL	•
### STANKE MAIN OFFEO2 **LINECNT=56.5172E=DDOOK** SUBRUUI 1AT MAIN OFFEO2**LINECNT=56.5172E=DDOOK** SUBRUUI 1AT MATULAL **NUUCK**LUAD**NAFFF CRS18480 LIMINSLUM ALS33,*VE31** C. A*V TUP 1F 15W **L** C. A*V TUP 1F 15W **L** C. A*V TUP 1F 15W **L** C. A*V TUP 1F 15W **L** C. A*V TUP 1F 15W **L** C. A*V TUP 1F 15W **L** C. A*V TUP 1F 15W **L** C. A*V TUP 1F 15W **L** C. A*V TUP 1F 15W **L** C. A*V TUP 1F 15W **L** C. A*V TUP 1F 15W **L** C. A*V TUP 1F 15W **L** C. A*V TUP 1F 15W **L** C. A*V TUP 1F 15W **L** C. A*V TUP 1F 15W **L** C. A*V TUP 1F 15W **L** C. A*V TUP 1F 15W **L** C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. A*V TUP 1F 15W **L** C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. A*V TUP 1F 15W **L** C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER TO P C. CANTANA CREATER	### SUBPLICITY KEALE ###################################	### SUBPLICER OFTIUMS - NAME: MAIN.UPT=02.LINECNT=56.SIZE=DODOK, SUBPLICER SUBPLITIONS - NAME: MAIN.UPT=02.LINECNT=56.SIZE=DODOK, SUBPLICITION A	
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UMPILER OPTIUNS - NAME= MAIN.UPT=02.LIMECNT=56.SIZE=DOOOK, SUBFOUTINE MATYECLEA.VP. 15W)	UMPILER OPTIUNS - NAME: MAIM, OPT=02, LIMECNT=56, SIZE=DOOOK, SUBRUUTINE MAYECLESCO, MAIN, OPT=02, LIMECNT=56, SIZE=DOOOK, SUBRUUTINE MAYECLESCO, MAIN, OPT=02, LIMECNT=56, SIZE=DOOOK, SUBRUUTINE MAYECLESCO, MAIN, OPT=02, LIMECNT=184) UMINON MAIN, OFT LIMECNTS, MAIN, OFT LIMECNTS, MAIN, OFT LIMECNTS, MAIN, OFT LIMECNTS, MAIN, OFT LIMECNTS, MAIN, OFT LIMECNTS, MAIN, OFT LIMECNTS, MAIN, OFT LIMECNTS, MAIN, OFT LIMECNTS, MAIN, OFT LIMECNTS, MAIN, OFT LIMECNTS, MAIN, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT LIMENS, OFT	C A** IUP 11 S. M. M. M. M. M. M. M. M. M. M. M. M. M.	**************************************
SUBFULIONS - NAME: MAIN-OPT-02-LINECNIT-56-51ZE=0000K+ SUBFULION	Subputive	UMPILER OPTIUNS - NAME: MAIN-UPT=02-LIMECNT=56.5IZE=D000K. SUURCULINE MAIN-UPT=02-LIMECR-LUAD-MAP-MUEDIT-ID-XREF SUURCULINE MEA-9-5-15W) IMPLICIT MEAL-66 (A-N-4-5-15W) UM-1455.UM A (3-3)-W(3)-P(3) C A+V IU P IS IS 2 0. ELSE AT** TO P UM-1455.UM A (3-3)-W(3)-P(3) SUM A C (-3-3) IFIISM 40-30-40 30 SUM A SUM-A(1-1)-W(K) C C CUNTINUE 10 P(1) = SUM-A(K-1)-W(K) END RETUKN RETUKN	******
SUBPULIAN TAYLELANDERS, LUAD, MAP, MUEDIT, 10, XR.F. CR.118+60 SUBPULIAN TAYLELAN, P. 1584) C. ARY 10 F 18 15M - 0, ELSE ATEW TO P. CR.118+00 C. ARY 10 J I = 1, 3 C. ARY 10 F 18 15M - 0, ELSE ATEW TO P. CR.118-500 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-50 C. CR. 118-5	SUBPULISE MATURES, LUAD, MAP, MUEDIT, 10, 7R.P. CR. 1840 SUBPULISE MATURES (A. 1840) LIMINATURE MATURES AT 10 P CR. 1840 C. ANY TO P. 18 18 18 18 18 18 18 18 18 18 18 18 18	SUBROUTINE MATERIANDE SUBROUTINE WATER TABLE SUBROUTINE MATERIAND TO THE SUBROUTINE MATERIAND TO THE SUBROUTINE MATERIAND TO THE SUBROUTINE WATER SUBBOUTINE TO THE SUBROUTINE SUBBOUTINE S	
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LIST OF SYMBOLS

SUBSTRUCTURE ANALYSIS

A	area; for stiffened panel, area of stiffener plus area of sheet corresponding to stiffener spacing, in.
8.	plate length, in.
As	area of stiffener, in. 2
A _w	area of web, in. ²
b _s	width of skin, in.
b _w	width of stiffener web, in.
С	constant (= 3.0)
c	number of cuts
đ	rivet diameter, in.
E	modulus of elasticity, psi
e,e _o	distance to neutral axis, in.
f	number of flanges
f _{cr}	crush strength, 1b
$\mathbf{f}_{\mathbf{w}}$	effective rivet off-set, in.
G	acceleration level
g	gravity term = 386.2 in./sec ²
Н	distance, in.
I	moment of inertia, in. 4
Io	moment of inertia about c.g., in.4
K	eccentricity factor
KE	kinetic energy, inlb
К _w	wrinkling coefficient
L , l	one-half column length, in.
ፒ '	effective length = L/\sqrt{C} , in.
М	effective moment, inlb

SUBSTRUCTURE ANALYSIS

$^{\mathtt{M}}_{\mathtt{P}}$	plastic hinge moment, in1b
N/A	neutral axis
N	effective axial force, lb
$^{ exttt{N}}_{ exttt{p}}$	plastic hinge axial force, lb.
p	rivet pitch, in.
P	applied axial force, lb
${ t P_f}$	monolithic failure load, 1b
$^{\mathtt{P}}_{\mathbf{w}}$	wrinkling failure load, 1b
q	distributed load, lb/in.
S	stopping distance, core radius of given cross section, in.
s	distance from midcross-section of skin to neutral axis of stiffener, in.
T	ratio of $\sigma/\sigma_{ m cy}$
T _{cr}	ratio of critical stress (σ_{cr}) to yield stress (σ_{cy})
^t c	minimum core thickness, in.
t _s	skin thickness, in.
t _w	stiffener thickness, in.
V	impact velocity, in./sec
M	weight, 1b
w _e	effective half width of skin, in.
y	lateral deflection
Z	function of column length and end shortening, in.
œ	defined in equation 51
η -	plasticity reduction factor
- Ij	cladding reduction factor
Π	$P_{i} = 3.14$
θ	tan ⁻¹ Z
ρ	radius of gyration = $\sqrt{I/A}$, in.
Υ	partial derivative

SUBSTRUCTURE ANALYSIS (Continued)

δu	incremental displacement
δ, Δ, u	displacement, end shortening
٧	Poisson's ratio
x	distance to c.g., in.
σ	compressive stress, psi
o _b	stress at outermost fiber of sheet skin, psi
σ co	column buckling stress, psi
o _{cr}	buckling stress, psi
σ c y	compressive yield stress, psi
ō cy	effective compressive yield stress, psi
ocyw	effective compressive yield stress for stiffener web, psi
°cy _s	effective compressive yield stress for skin, psi
o _e	Euler stress, psi
°f	crippling (or failure) stress, psi
o _{fr}	failure stress of riveted panel, psi
o W	failure stress due to wrinkling mode, psi
o zo	buckling stress at $L'/\rho = 20$
Subscripts:	
ъ	bending
f	filler
i	i th segment
max.	maximum
min.	minimum
p	plastic state
ន	skin or sheet
W	stiffener web

SUBSTRUCTURE ANALYSIS (Continued)

Constants (function of stiffened panel configuration)

 $\beta_{\mathbf{f}}$

PROGRAM "KPASH"

rotational transformation matrix from body axes to Ai

ground axes

 A_i^T transpose of A, matrix

CE total crash spring (external spring) energy absorbed

DE total damping energy dissipated

DRI dynamic response index

 $\mathbf{E}_{\mathtt{TOT}}$ total system energy

internal beam damping force (or moment) for the ij th beam in the ℓ^{th} direction $^{\mathtt{FD}}\mathtt{ij}\ell$

 $\mathtt{FSP}_{\mathtt{ijk}}$ crash spring forces; spring ij in the kth direction

 ${\tt F}_{\tt ij\ell}$ force (or moment) at point j due to beam ij, in the

lth direction

moments of inertia of lumped mass m, about ith body I_{xi}, I_{yi}, I_{zi}

fixed axes

product of inertia of lumped mass m, about ith body I_{xyi}, I_{yzi}, I_{zxi}

axes

KE total kinetic energy

length of vector from m, to ground contact point C'ik $\mathbf{l}_{\mathbf{i}\mathbf{k}}$

ith lumped mass m_i

number of lumped masses N

total potential energy PE

SE total strain energy absorbed

TERM term in expression for crash spring energy

PROGRAM "KRASH" (Continued)

W; weight of ith lumped mass

 x_i, y_i, z_i ground coordinates of m_i

u_i, v_i, w_i ith body axes component of absolute translational velocity vector of mass i

p_i, q_i, r_i ith body axes components of absolute angular velocity vector of mass i

 $\Delta x_i', \Delta y_i', \Delta z_i' = \{\Delta vc_i\},\$ incremental displacement of point i, ith body axes $\Delta inp_i, \Delta inq_i, \Delta inr_i \quad incremental \ rotation \ of \ point \ i, \ ith \ body \ axes$

incremental displacement vector of point j with respect to point i, due to deformation of beam ij, in the ℓ^{th} direction

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A_i , B_i , C_i	Terms used in Euler's equations of motion
[A]	Rotation transformation matrix from body axes to ground axes
$[\overline{\mathtt{A}}_{\mathtt{i}}]$	Matrix relating (p_i, e_i, ψ_i) to (p_i, q_i, r_i) in equation (92)
[Å ₁]	Time derivative of [Ai]
[A"]	Rotation transformation matrix from ith body axes to c.g. axes
[A ₁ j]	Rotation transformation matrix from beam ij axes to body i axes
[4]	Rotation transformation matrix from c.g. axes to ground axes
[•Ā]	Matrix relating (p', Φ', ψ') to (p', q', r') in equation (88)
C _{ik}	End point of kth spring on ith mass
C _{1k}	Ground contact point of kth spring on ith mass
dvcijk	Ground axes components of wector from mi to Cik
dvc _{ijk}	Ground axes components of vector for C_{1k}^{\dagger} to C_{1k}
	Derivative matrix
[p']	Derivative matrix
^{FM} ijkl	Running time sum of ΔFM_{ijkl}
FM ₁ jkl	Value of FM _{ijkl} at time of loading reversal
FSP_{ijk}	Body i axes components of spring force at ground contact point $C_{\mbox{i}\mbox{k}}$
FSPO _{ik}	Axial compressive force in kth spring on ith mass
FSFO _{ik}	Value of FSPO _{ik} at time of loading reversal

$\mathtt{FSPO}_{\mathtt{Fik}}$	Final value of $FSPO_{ik}$ in input table of s_{ik} vs. $FSPO_{ik}$
G	Center-of-gravity of total vehicle
Н	Origin of helicopter coordinate system (F.S.O, B.L.O, W.L.O)
He _{xi} , He _{yi} , He _{zi}	Angular momenta of mi due to rotation of masses internal to $m_{\dot{i}}$
Ixi, Iyi, Tzi	Moments of inertia of lumped mass mi, about ith body fixed axes
I _{xyi} , I _{yzi} , I _{zxi}	Products of inertia of lumped mass m ₁ , about ith body fixed axes
ke _{ik}	Linear unloading stiffness for kth spring
$[\kappa_{i,j}]$	Six by six linear stiffness matrix for beam ij
[KR _{ij}]	Six by six diagonal stiffness reduction matrix for beam ij
1 _{ik}	Length of vector from m ₁ to ground contact point C _{ik}
$\overline{1}_{xi}$, $\overline{1}_{yi}$, $\overline{1}_{zi}(\overline{1}_{ik})$	Free length of kth spring on ith mass
1 _{ci}	Aerodynamic lift constant
LIFT ₁	Aerodynamic lift on m1, positive up, in ground axes
m _i	ith lumped mass
mu _{ik}	Ground-spring friction coefficient for kth spring on ith mass
N	Total number of lumped masses
$\overline{\mathfrak{n}}_{\mathtt{ik}}$	Unit vector triad fixed in ith body coordinate system
\overline{n}_x , \overline{n}_y , \overline{n}_z	Unit vector triad fixed in ground coordinate system
0	Origin of ground coordinate system

p _i , q _i , r _i	ith body axes components of absolute angular velocity vector of mass i
p', q', r'	c.g. axes components of initial(t=0) vehicle angular velocity vector
[Pl1]	Contact point velocity matrix used in equation (60)
s ik	Axial external spring compression, kth spring on ith mass
5ik	Value of sik at time of loading reversal
*Fik	Final value of sik in input table of sik vs. FSPOik
#1k	kth spring axial compression measured relative to current load stroke curve origin
and ik	Horizontal shift of s_{1k}^{\prime} coordinates with respect to s_{1k} coordinates
t	Time
[TiJ]	Static balance matrix used in equation (30b)
u _i , v _i , v _i	Body i axes components of absolute translational velocity vector of point m ₁
va _{i,j}	x_i, y_i, z_i
vb _{ij}	Running time sum of Δvb_{ij}
\overline{vb}_{ijl}	Value of vb _{ijl} at time of loading reversal
v _{ik}	Magnitude of ground plane contact point velocity
vbijl	1th total beam deflection measured relative to current load-stroke curve origin
vb"jl	Horizontal shift of vb _{ijl} coordinates with respect to vb _{ijl} coordinates
vc_{ijk}	Ground coordinates of point Cik

vep _{ijk}	Ground axes components of absolute velocity of ground contact point C_{1k}
M C	Velocity vector of Cik with respect to mi
o de,	Velocity vector of Cik with respect to ground
o √ mi	Velocity vector of ma with respect to ground
Wi	Weight of ith lumped mass
WTOT	Total vehicle weight
xb _{ij} , yb _{ij} , zb _{ij}	Beam ij coordinates
x _G , y _G , z _G (vg _j)	Ground coordinates of initial (t = o) c.g. position
x _G , y _G , z _G	Ground axes components of initial (t = o) c.g. velocity vector
x _G , y _G , z _G (vgpp _{ij})	Helicopter axes coordinates of vehicle c.g. (point G)
$x_i, y_i, z_i (va_{ij})$	Ground coordinates of mi
$x_{1}^{1}, y_{1}^{1}, z_{1}^{1} (vip_{1j})$	Coordinates of m _i in center-of-gravity coordinate system
x", y", z" (vipp _{ij})	Coordinates of m ₁ in helicopter coordinate system
x _{1j} , y _{1j} , z _{ij}	Ground coordinates of vector from point i to point j
x _{ij} , y _{ij} , z _{ij}	ith body coordinates of vector from point i to point j
$\begin{pmatrix} X_{1J}^{i}, & Y_{1J}^{i}, & Z_{1J}^{i} \\ L_{1J}^{i}, & M_{1J}^{i}, & N_{1J}^{i} \end{pmatrix}$	Total (summed over time) internal forces and moments at point i due to beam ij, ith body axes
$\begin{pmatrix} X_{ji}^{i}, Y_{ji}^{i}, Z_{ji}^{i} \\ L_{ji}^{i}, M_{ji}^{i}, N_{ji}^{i} \end{pmatrix}$	Total (summed over time) internal forces and moments at point j due to beam ij, j th body axes

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$\begin{pmatrix} X_{i}, Y_{i}, Z_{i} \\ L_{i}, M_{i}, N_{i} \end{pmatrix}$	Total forces and moments on mass i, in ith body axes
X _{A1} Y _{A1} , Z _{A1}	Aerodynamic forces, ith body axes
(XCi, YCi, ZCi) LCi, MCi, NCi)	Crash (external) forces and moments, ith body axes
X _{Gi} , Y _{Gi} , Z _{Gi}	Gravity forces, ith body axes
$\begin{pmatrix} x_{\text{Ii}}, & Y_{\text{Ii}}, & Z_{\text{Ii}} \\ L_{\text{II}}, & M_{\text{II}}, & N_{\text{Ii}} \end{pmatrix}$	Internal forces and moments, ith body axes
$\mathtt{xvoc}_{\mathtt{ijk}}$	Ground axes components of spring force at ground contact point Cik, positive up, left and aft
ZC MAX	Vertical distance from c.g. to lowest Cik
Δi	Determinate expression used in equation (68)
Δ F _{i,jk}	Incremental forces and moments at point j due to beam ij
$oldsymbol{\Delta}_{ extsf{FM}_{ extsf{ijkl}}}$	kth incremental load due to 1th incremental deflection for beam ij
$\Delta \phi_{ ext{b}_{ ext{i}, ext{j}}}$, $\Delta \phi_{ ext{b}_{ ext{i}, ext{j}}}$, $\Delta \gamma_{ ext{b}_{ ext{i}, ext{j}}}$	Incremental rotations of point j with respect to point i, in beam ij axes
$\Delta \phi_i, \Delta \phi_i, \Delta \psi_i$	Incremental change in ith mass Euler angles
Δt	Numerical integration time interval
$\Delta_{{ t vb}_{f i,j}}$	Six element vector made up of Δxb_{ij} , Δyb_{ij} , Δzb_{ij} , Δb_{ij}
Δvb _{i,j}	Incremental displacement vector of point j with respect to point i, due to deformation of beam ij
$\Delta vd_{i,i}$	Incremental displacement vector of point j with

respect to point i

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Δ vr _{i,j}	Incremental displacement vector of point j with respect to point i, due to rotation of mass i
Δ xb _{ij} , Δ yb _{ij} , Δ zb _{ij}	Coordinates of $\Delta \forall b_{ij}$ in beam ij axes
Δx_i , Δy_i , Δz_i	Incremental displacement of point i, ground axes
$\Delta_{x_{ij}}$, $\Delta_{y_{ij}}$, $\Delta_{z_{ij}}$	Incremental displacement of point j with respect to point i in ground axes
$\begin{pmatrix} \Delta X_{i,j}, \Delta Y_{i,j}, \Delta Z_{i,j} \\ \Delta L_{i,j}, \Delta M_{i,j}, \Delta N_{i,j} \end{pmatrix}$	Incremental internal forces and moments at point j due to beam ij, in beam ij axes (elements of ΔF_{ij} vector)
$\begin{pmatrix} \Delta X_{i,j}^{O}, \Delta Y_{i,j}^{O}, \Delta Z_{i,j}^{O} \\ \Delta L_{i,j}^{O}, \Delta M_{i,j}^{O}, \Delta N_{i,j}^{O} \end{pmatrix}$	Incremental internal forces and moments at point j due to beam ij, ground axes
$\left(\frac{\overline{\Delta x_{\hat{Y}_j}},\overline{\Delta Y_{\hat{Y}_j}},\overline{\Delta Z_{\hat{Y}_j}}}{\overline{\Delta M_{\hat{Y}_j}},\overline{\Delta M_{\hat{Y}_j}}}\right)$	Incremental internal forces and moments at point i due to beam ij ,ground axes
$\begin{pmatrix} \Delta x_{ij}^{i}, \Delta y_{ij}^{i}, \Delta z_{ij}^{i} \\ \Delta L_{ij}^{i}, \Delta M_{ij}^{i}, \Delta N_{ij}^{i} \end{pmatrix}$	Incremental internal forces and moments at point i due to beam ij, i th body axes
$\begin{pmatrix} \Delta x_{ji}^{\prime}, \Delta Y_{ji}^{\prime}, \Delta Z_{ji}^{\prime} \\ \Delta L_{ji}^{\prime}, \Delta M_{ji}^{\prime}, \Delta N_{ji}^{\prime} \end{pmatrix}$	Incremental internal forces and moments at point j due to beam ij , j th body axes
ø _i , 🚓 //i	Euler angles from ground axes to body axes (time varying)
$\phi_{ij}, \phi_{ij}, \gamma_{ij}$	Euler angles from ith body axes to beam ij axes (constant)
ø', e', y' '	Euler angles from ground axes to c.g. axes (constant); initial (t=o)-attitude of vehicle

U.S. Government Printing Office: 1974-636-037/1G Region No. 3-11

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 $\phi_{\mathbf{i}}^{\shortparallel},\ \boldsymbol{\phi}_{\mathbf{i}}^{\shortparallel},\ \boldsymbol{\gamma}_{\mathbf{i}}^{\shortparallel}$

Euler angles from c.g. axes to ith body axes (con-